

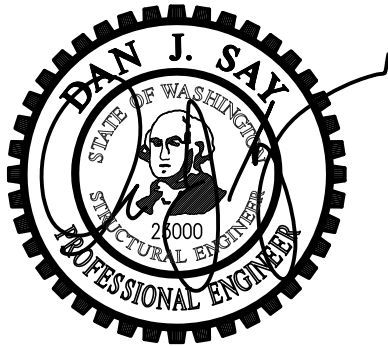


Supplemental Structural Calculations for:

Huber Residence

9611 SE 72nd St

Mercer Island, WA 98040



Prepared for: Brandt Design Group

Job #: 01519-2021-06

Date: April 19, 2022

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall

Code Reference:

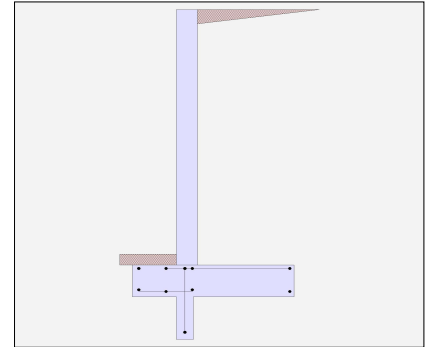
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

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Design Summary

Wall Stability Ratios

Overturning	=	2.38	OK
Sliding	=	1.54	OK
Global Stability	=	1.87	
Total Bearing Load	=	8,893 lbs	
...resultant ecc.	=	11.76 in	
Soil Pressure @ Toe	=	2,656 psf	OK
Soil Pressure @ Heel	=	116 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	3,718 psf	
ACI Factored @ Heel	=	162 psf	
Footing Shear @ Toe	=	10.0 psi	OK
Footing Shear @ Heel	=	17.4 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	3,189.4 lbs	
less 100% Passive Force	=	- 2,250.0 lbs	
less 100% Friction Force	=	- 2,667.8 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	10.00
Rebar Size	=	# 7
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.676

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	4,032.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	16,128.0

Moment.....Allowable = 23,826.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	44.4

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 125.0

Rebar Depth 'd' in = 7.56

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.4948 in2/ft		
(4/3) * As :	0.6597 in2/ft	Min Stem T&S Reinf Area 2.880 in2	
200bd/fy : 200(12)(7.5625)/60000 :	0.3025 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft	
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.4948 in2/ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.8 in2/ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0245 in2/ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	4.67
Total Footing Width	=	6.42
Footing Thickness	=	18.00 in
Key Width	=	8.00 in
Key Depth	=	24.00 in
Key Distance from Toe	=	1.75 ft
f'c = 2,500 psi	Fy = 60,000 psi	
Footing Concrete Density = 150.00 pcf		
Min. As % = 0.0018		
Cover @ Top 2.00	@ Btm= 3.00 in	

Footing Design Results

		Toe	Heel
Factored Pressure	=	3,718	162 psf
Mu' : Upward	=	5,199	6,393 ft-#
Mu' : Downward	=	528	15,203 ft-#
Mu: Design	=	4,670	8,810 ft-#
phiMn	=	37,413	29,913 ft-#
Actual 1-Way Shear	=	9.96	17.42 psi
Allow 1-Way Shear	=	75.00	75.00 psi
Toe Reinforcing	=	# 7 @ 12.00 in	
Heel Reinforcing	=	# 6 @ 12.00 in	
Key Reinforcing	=	# 4 @ 10.55 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	2.49 in2
Min footing T&S reinf Area per foot	0.39 in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 6.17 in	#4@ 12.35 in
#5@ 9.57 in	#5@ 19.14 in
#6@ 13.58 in	#6@ 27.16 in

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Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	3,189.4	4.50	14,352.2	Soil Over HL (ab. water tbl)	5,749.0	4.50	25,868.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.50	25,868.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.88	
				Surcharge Over Toe =			
				Stem Weight(s) =	1,500.0	2.17	3,250.0
				Earth @ Stem Transitions =			
Total	= 3,189.4	O.T.M. =	14,352.2	Footing Weight =	1,443.6	3.21	4,631.1
				Key Weight =	200.0	2.08	416.7
				Vert. Component =			
Resisting/Overturning Ratio		= 2.38		Total =	8,892.6 lbs	R.M.=	34,166.3
Vertical Loads used for Soil Pressure =		8,892.6 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.138 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #7 bar specified in this stem design segment = 40.95 in

Development length for #7 bar specified in this stem design segment = 31.50 in

Hooked embedment length into footing for #7 bar specified in this stem design segment = 9.09 in

As Provided = 0.8000 in²/ft

As Required = 0.4948 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

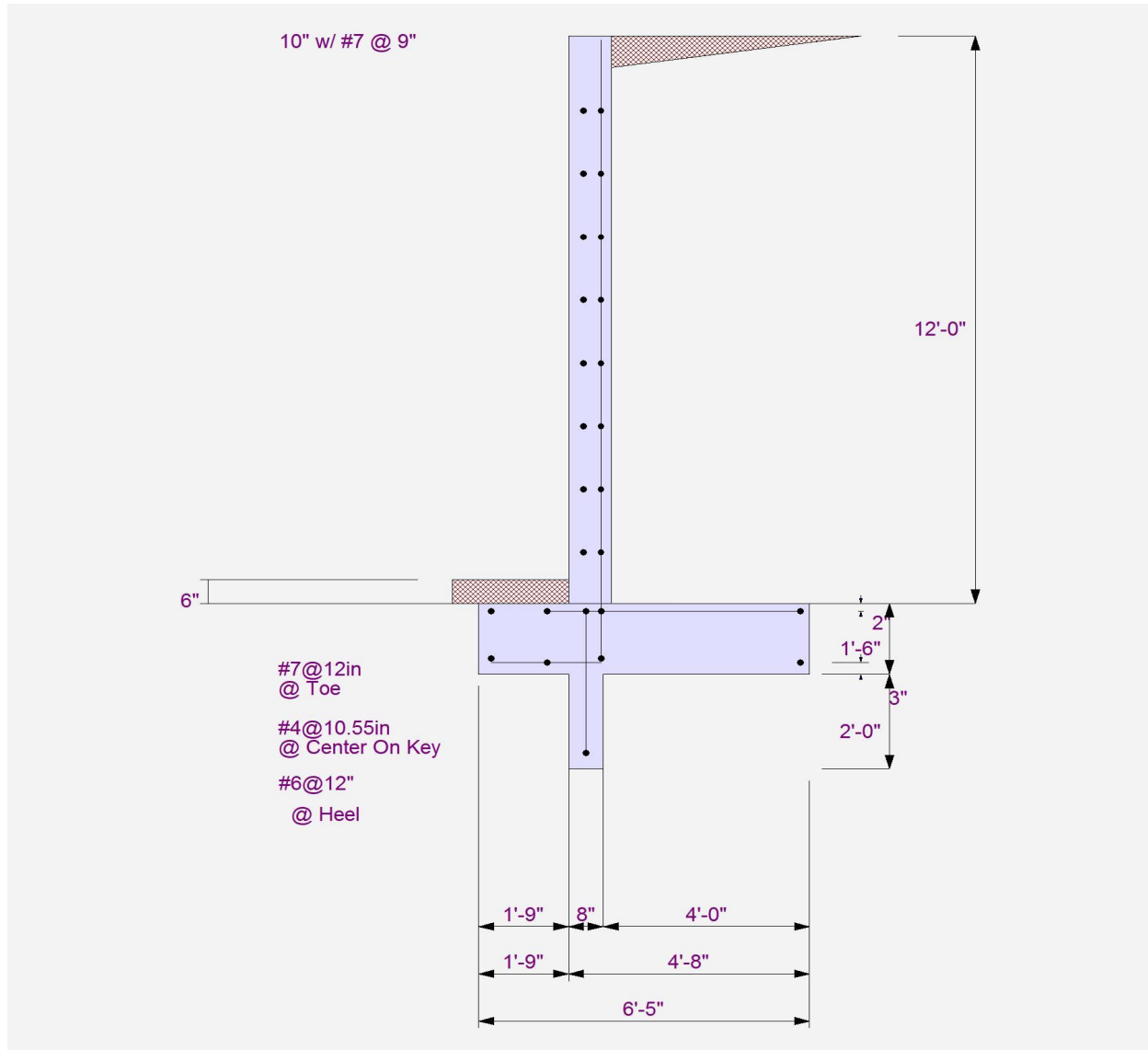
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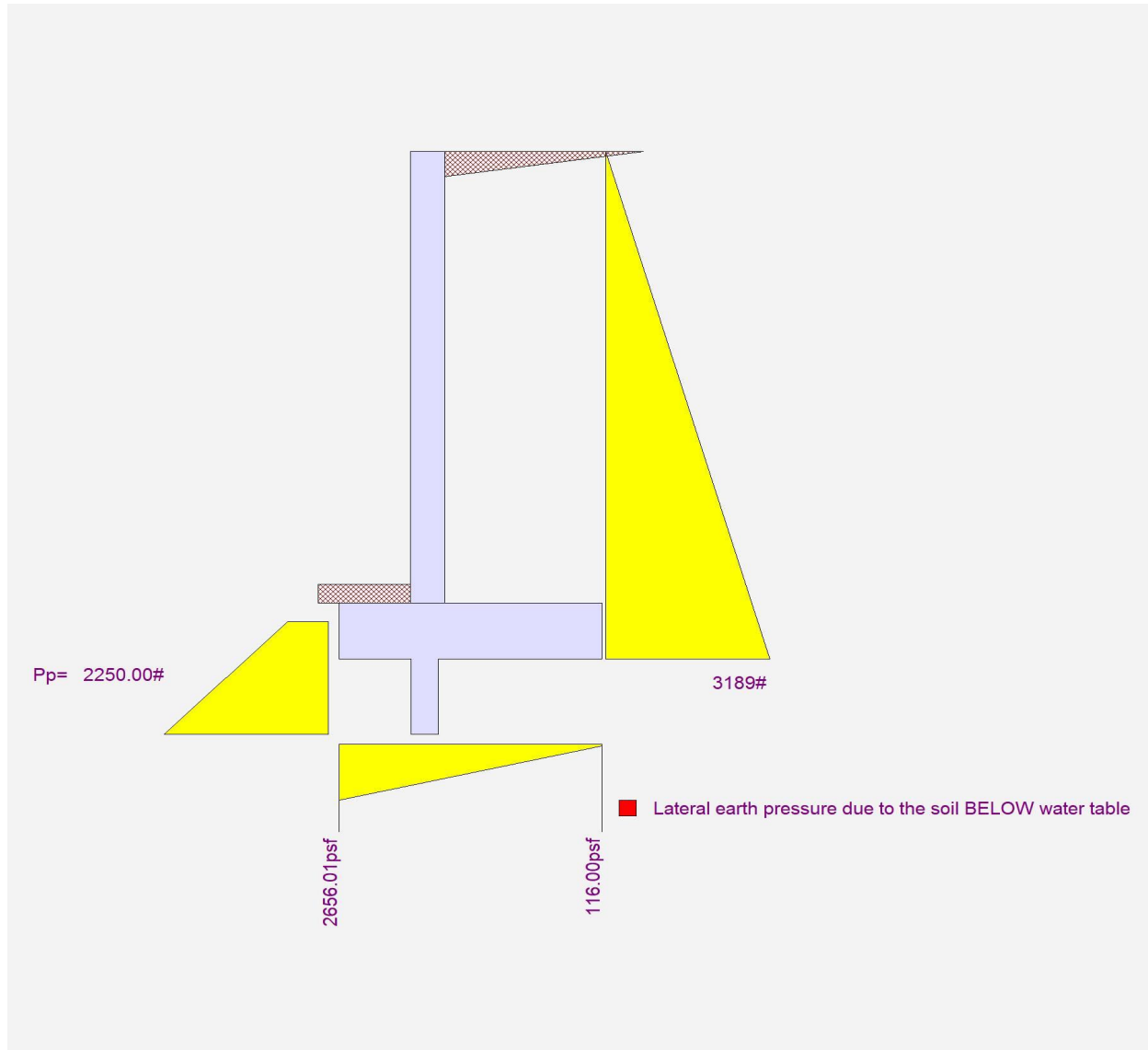
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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall W/ Seismic

Code Reference:

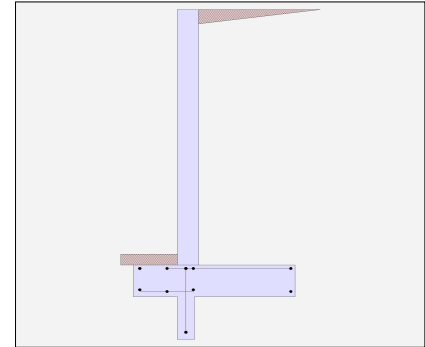
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footings Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	81.000
Total Seismic Force	=	1,093.500

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

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Design Summary

Wall Stability Ratios

Overturning	=	1.75	OK
Sliding	=	1.24	Ratio < 1.5!
Global Stability	=	1.87	
Total Bearing Load	=	8,893	lbs
...resultant ecc.	=	18.73	in
Soil Pressure @ Toe	=	3,599	psf OK
Soil Pressure @ Heel	=	0	psf OK
Allowable	=	4,000	psf
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	5,039	psf
ACI Factored @ Heel	=	0	psf
Footing Shear @ Toe	=	13.7	psi OK
Footing Shear @ Heel	=	27.4	psi OK
Allowable	=	75.0	psi

Sliding Calcs

Lateral Sliding Force	=	3,954.8	lbs
less 100% Passive Force	-	2,250.0	lbs
less 100% Friction Force	= -	2,667.8	lbs
Added Force Req'd	=	0.0	lbs OK
....for 1.5 Stability	=	1,014.5	lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	10.00
Rebar Size	=	# 7
Rebar Spacing	=	9.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.921
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Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	5,004.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	21,960.0

Moment.....Allowable	=	23,826.6
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Shear.....Actual

Service Level	psi =	
Strength Level	psi =	55.1

Shear.....Allowable	psi =	75.0
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Anet (Masonry)	in2 =	
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Wall Weight	psf =	125.0
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Rebar Depth 'd'	in =	7.56
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Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall W/ Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.6737 in2/ft		
(4/3) * As :	0.8982 in2/ft	Min Stem T&S Reinf Area 2.880 in2	
200bd/fy : 200(12)(7.5625)/60000 :	0.3025 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft	
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.6737 in2/ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.8 in2/ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0245 in2/ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	4.67
Total Footing Width	=	6.42
Footing Thickness	=	18.00 in
Key Width	=	8.00 in
Key Depth	=	24.00 in
Key Distance from Toe	=	1.75 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm= 3.00 in

Footing Design Results

		Toe	Heel
Factored Pressure	=	5,039	0 psf
Mu' : Upward	=	6,805	2,228 ft-#
Mu' : Downward	=	528	15,203 ft-#
Mu: Design	=	6,277	12,975 ft-#
phiMn	=	37,413	29,913 ft-#
Actual 1-Way Shear	=	13.67	27.41 psi
Allow 1-Way Shear	=	75.00	75.00 psi
Toe Reinforcing	=	# 7 @ 12.00 in	
Heel Reinforcing	=	# 6 @ 12.00 in	
Key Reinforcing	=	# 4 @ 9.12 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	2.49	in2
Min footing T&S reinf Area per foot	0.39	in2 /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 6.17 in		#4@ 12.35 in
#5@ 9.57 in		#5@ 19.14 in
#6@ 13.58 in		#6@ 27.16 in

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Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	3,189.4	4.50	14,352.2	Soil Over HL (ab. water tbl)	5,749.0	4.50	25,868.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.50	25,868.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.88	
Seismic Earth Load =	765.5	6.75	5,166.8	Surcharge Over Toe =			
=				Stem Weight(s) =	1,500.0	2.17	3,250.0
Total =	3,954.8	O.T.M. =	19,519.0	Earth @ Stem Transitions =			
				Footing Weight =	1,443.6	3.21	4,631.1
				Key Weight =	200.0	2.08	416.7
				Vert. Component =			
				Total =	8,892.6 lbs	R.M.=	34,166.3

Resisting/Overturning Ratio = **1.75**
 Vertical Loads used for Soil Pressure = 8,892.6 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.187 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall W/ Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #7 bar specified in this stem design segment = 40.95 in

Development length for #7 bar specified in this stem design segment = 31.50 in

Hooked embedment length into footing for #7 bar specified in this stem design segment = 12.38 in

As Provided = 0.8000 in/ft

As Required = 0.6737 in/ft

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Cantilevered Retaining Wall

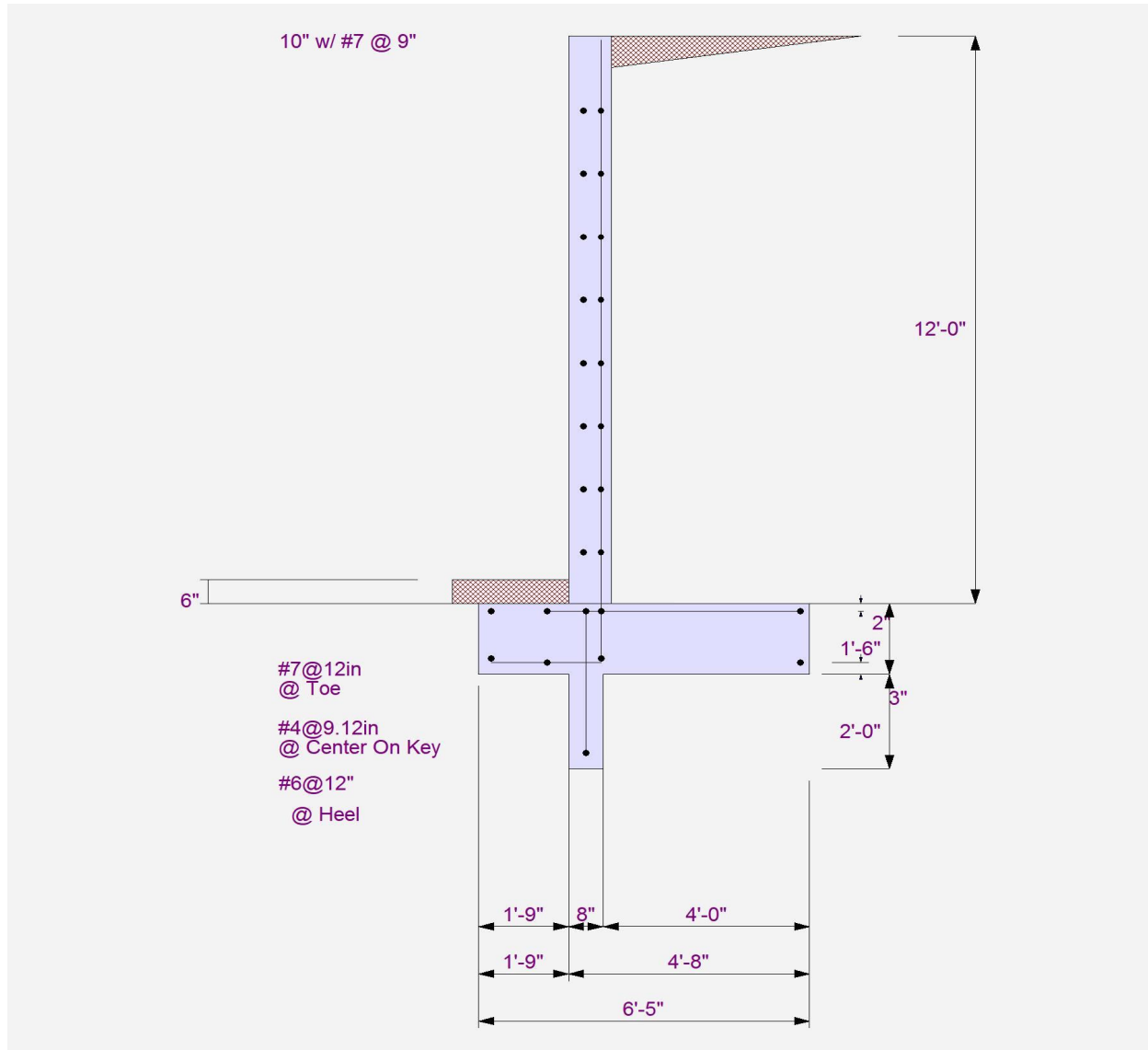
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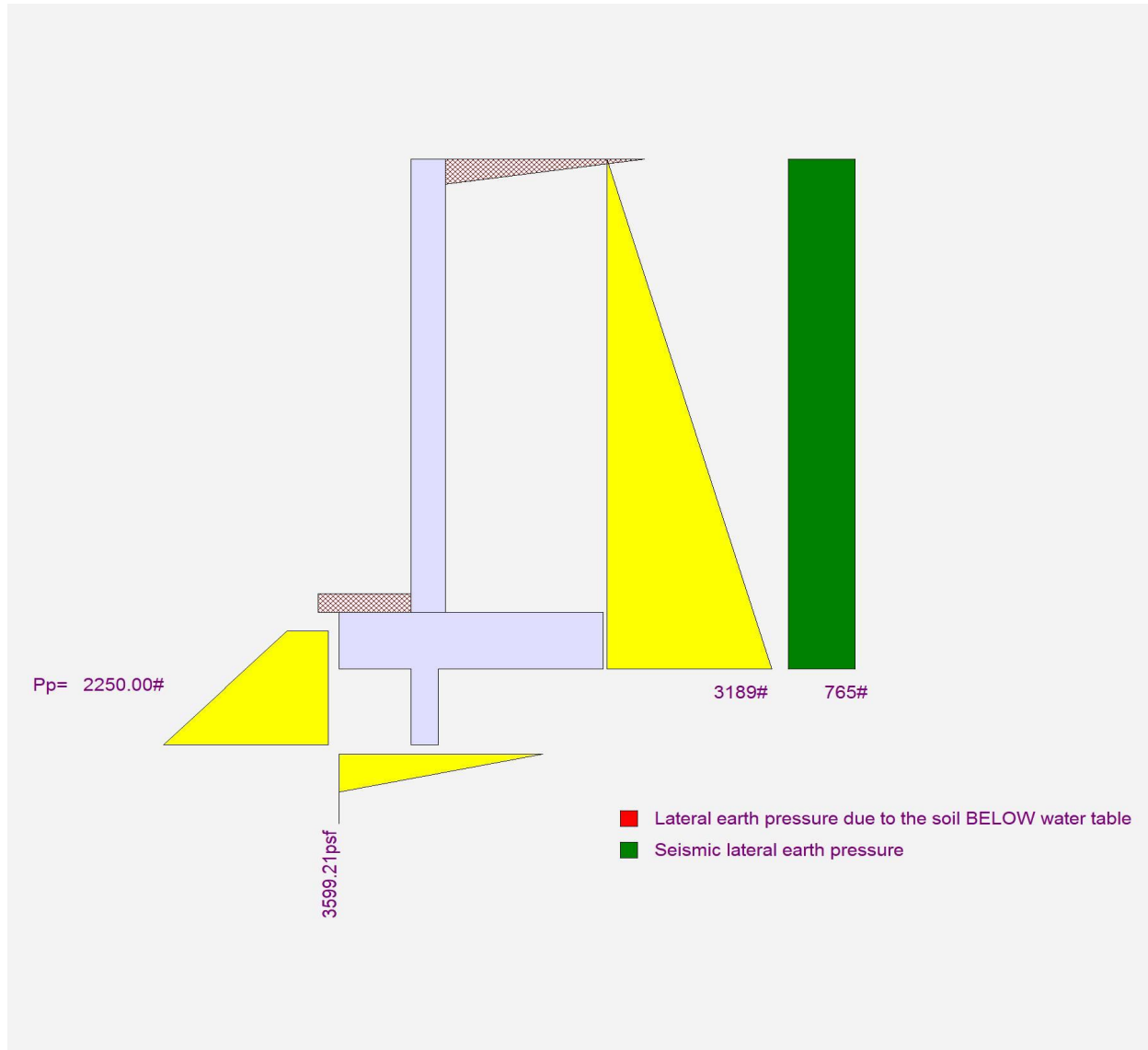
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SWENSON SAY FAGET

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DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall

Code Reference:

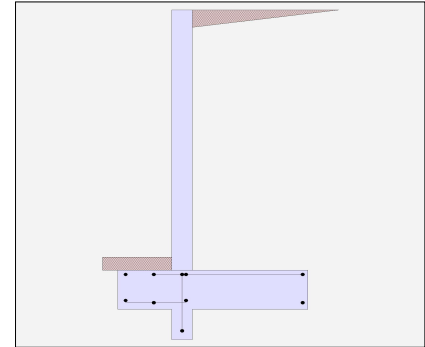
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall

Design Summary

Wall Stability Ratios

Overturning	=	3.01	OK
Sliding	=	1.52	OK
Global Stability	=	2.07	
Total Bearing Load	=	7,191 lbs	
...resultant ecc.	=	7.22 in	
Soil Pressure @ Toe	=	1,849 psf	OK
Soil Pressure @ Heel	=	484 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,588 psf	
ACI Factored @ Heel	=	677 psf	
Footing Shear @ Toe	=	6.6 psi	OK
Footing Shear @ Heel	=	10.3 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	2,314.4 lbs	
less 100% Passive Force	-	1,354.2 lbs	
less 100% Friction Force	= -	2,157.2 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 7
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.712
---------------	---	-------

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,800.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	9,333.3

Moment.....Allowable

=	13,107.2
---	----------

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	41.9

Shear.....Allowable

psi =	75.0
-------	------

Anet (Masonry)

in2 =	
-------	--

Wall Weight

psf =	100.0
-------	-------

Rebar Depth 'd'

in =	5.56
------	------

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.3955 in2/ft		
(4/3) * As :	0.5274 in2/ft	Min Stem T&S Reinf Area 1.920 in2	
200bd/fy : 200(12)(5.5625)/60000 :	0.2225 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.3955 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.6 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.7535 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	4.42
Total Footing Width	=	6.17
Footing Thickness	=	18.00 in
Key Width	=	8.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.75 ft
f'c = 2,500 psi	Fy = 60,000 psi	
Footing Concrete Density = 150.00 pcf		
Min. As % = 0.0018		
Cover @ Top 2.00	@ Btm= 3.00 in	

Footing Design Results

		Toe	Heel
Factored Pressure	=	2,588	677 psf
Mu' : Upward	=	3,686	7,482 ft-#
Mu' : Downward	=	528	12,441 ft-#
Mu: Design	=	3,158	4,959 ft-#
phiMn	=	37,413	29,913 ft-#
Actual 1-Way Shear	=	6.64	10.32 psi
Allow 1-Way Shear	=	75.00	75.00 psi
Toe Reinforcing	=	# 7 @ 12.00 in	
Heel Reinforcing	=	# 6 @ 12.00 in	
Key Reinforcing	=	# 4 @ 13.89 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	2.40 in2
Min footing T&S reinf Area per foot	0.39 in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 6.17 in	#4@ 12.35 in
#5@ 9.57 in	#5@ 19.14 in
#6@ 13.58 in	#6@ 27.16 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,314.4	3.83	8,871.8	Soil Over HL (ab. water tbl)	4,686.7	4.29	20,112.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.29	20,112.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.88	
				Surcharge Over Toe =			
				Stem Weight(s) =	1,000.0	2.08	2,083.3
				Earth @ Stem Transitions =			
Total	= 2,314.4	O.T.M.	= 8,871.8	Footing Weight =	1,387.4	3.08	4,277.2
				Key Weight =	116.7	2.08	243.1
				Vert. Component =			
Resisting/Overturning Ratio		=	3.01	Total =	7,190.7 lbs	R.M.=	26,715.6
Vertical Loads used for Soil Pressure =		7,190.7 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.083 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #7 bar specified in this stem design segment = 40.95 in

Development length for #7 bar specified in this stem design segment = 31.50 in

Hooked embedment length into footing for #7 bar specified in this stem design segment = 9.69 in

As Provided = 0.6000 in/ft

As Required = 0.3955 in/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

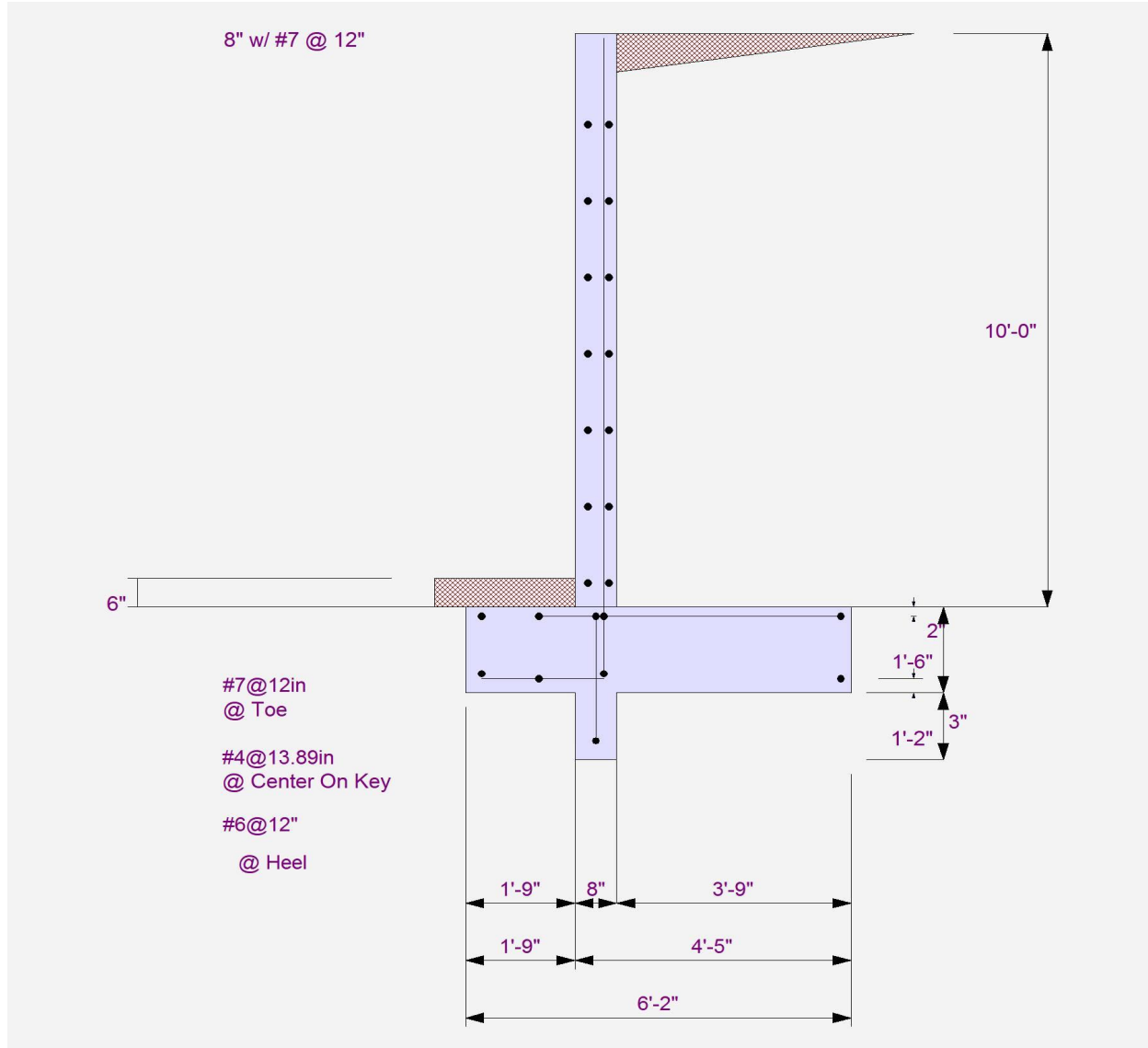
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

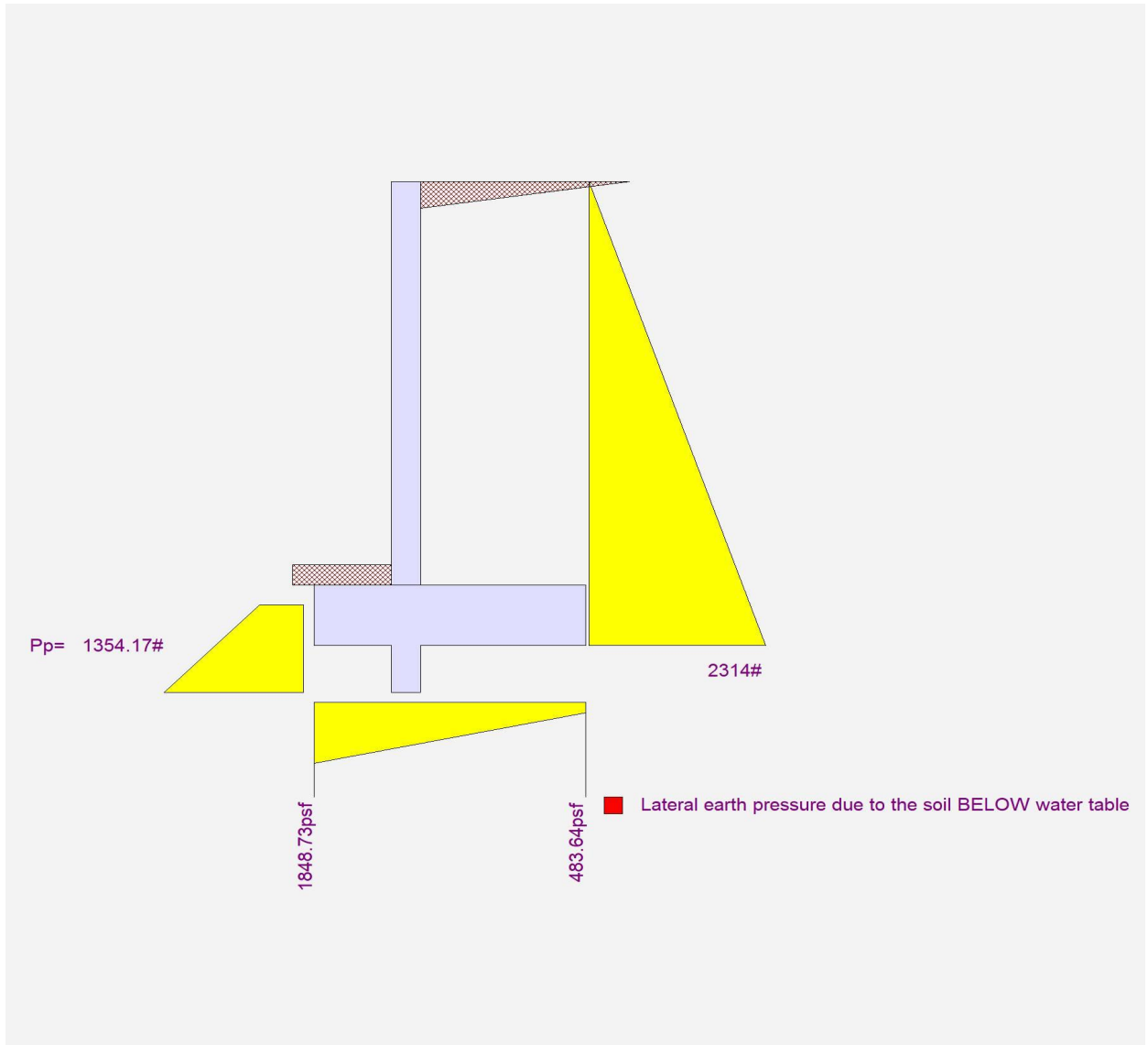
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic

Code Reference:

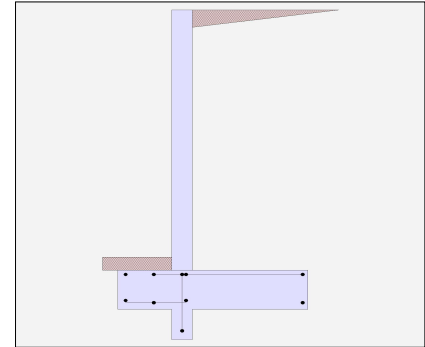
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	10.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Uniform Seismic Force	=	69.000
Total Seismic Force	=	793.500

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic

Design Summary

Wall Stability Ratios

Overturning	=	2.21	OK
Sliding	=	1.22	Ratio < 1.5!
Global Stability	=	2.07	
Total Bearing Load	=	7,191	lbs
...resultant ecc.	=	12.55	in
Soil Pressure @ Toe	=	2,353	psf OK
Soil Pressure @ Heel	=	0	psf OK
Allowable	=	4,000	psf
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	3,294	psf
ACI Factored @ Heel	=	0	psf
Footing Shear @ Toe	=	8.6	psi OK
Footing Shear @ Heel	=	15.9	psi OK
Allowable	=	75.0	psi

Sliding Calcs

Lateral Sliding Force	=	2,869.8	lbs
less 100% Passive Force	-	1,354.2	lbs
less 100% Friction Force	= -	2,157.2	lbs
Added Force Req'd	=	0.0	lbs OK
....for 1.5 Stability	=	793.4	lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 7
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.975

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	3,490.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	12,783.3

Moment.....Allowable = 13,107.2

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	52.3

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 100.0

Rebar Depth 'd' in = 5.56

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.5417 in ² /ft		
(4/3) * As :	0.7223 in ² /ft	Min Stem T&S Reinf Area 1.920 in ²	
200bd/fy : 200(12)(5.5625)/60000 :	0.2225 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.5417 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.6 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.7535 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.75 ft
Heel Width	=	4.42
Total Footing Width	=	6.17
Footing Thickness	=	18.00 in
Key Width	=	8.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.75 ft
f'c = 2,500 psi	Fy = 60,000 psi	
Footing Concrete Density = 150.00 pcf		
Min. As % = 0.0018		
Cover @ Top 2.00	@ Btm = 3.00 in	

Footing Design Results

	Toe	Heel
Factored Pressure	= 3,294	0 psf
Mu' : Upward	= 4,563	4,533 ft-#
Mu' : Downward	= 528	12,441 ft-#
Mu: Design	= 4,034	7,908 ft-#
phiMn	= 37,413	29,913 ft-#
Actual 1-Way Shear	= 8.65	15.89 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 7 @ 12.00 in	
Heel Reinforcing	= # 6 @ 12.00 in	
Key Reinforcing	= # 4 @ 13.89 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	2.40 in ²
Min footing T&S reinf Area per foot	0.39 in ² /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 6.17 in	#4@ 12.35 in
#5@ 9.57 in	#5@ 19.14 in
#6@ 13.58 in	#6@ 27.16 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	2,314.4	3.83	8,871.8	Soil Over HL (ab. water tbl)	4,686.7	4.29	20,112.0
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		4.29	20,112.0
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.88	
Seismic Earth Load =	555.5	5.75	3,193.8	Surcharge Over Toe =			
=				Stem Weight(s) =	1,000.0	2.08	2,083.3
Total =	2,869.8	O.T.M. =	12,065.6	Earth @ Stem Transitions =			
				Footing Weight =	1,387.4	3.08	4,277.2
				Key Weight =	116.7	2.08	243.1
				Vert. Component =			
				Total =	7,190.7 lbs	R.M.=	26,715.6

Resisting/Overturning Ratio = **2.21**
 Vertical Loads used for Soil Pressure = 7,190.7 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.106 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #7 bar specified in this stem design segment = 40.95 in

Development length for #7 bar specified in this stem design segment = 31.50 in

Hooked embedment length into footing for #7 bar specified in this stem design segment = 13.27 in

As Provided = 0.6000 in²/ft

As Required = 0.5417 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

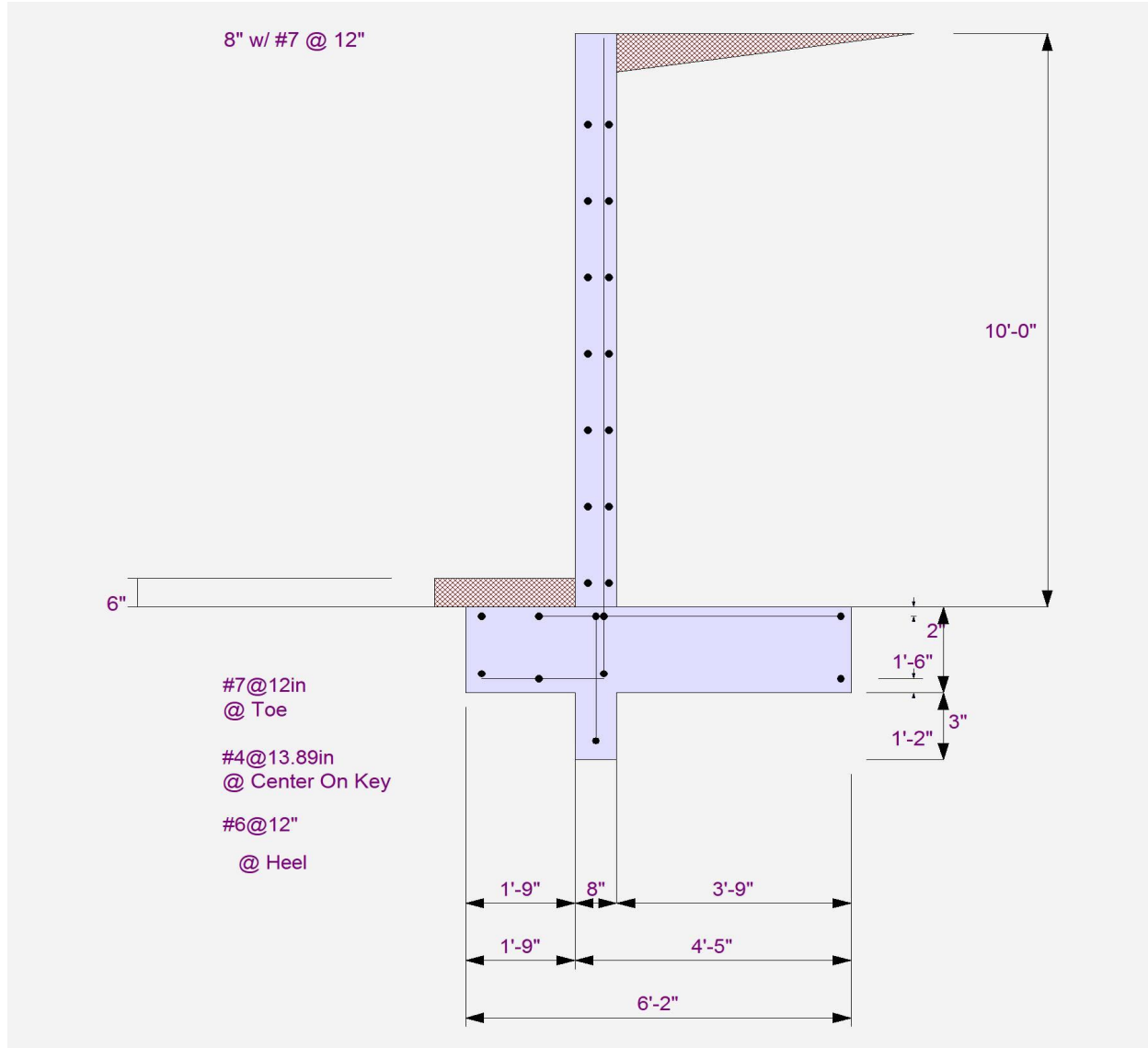
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

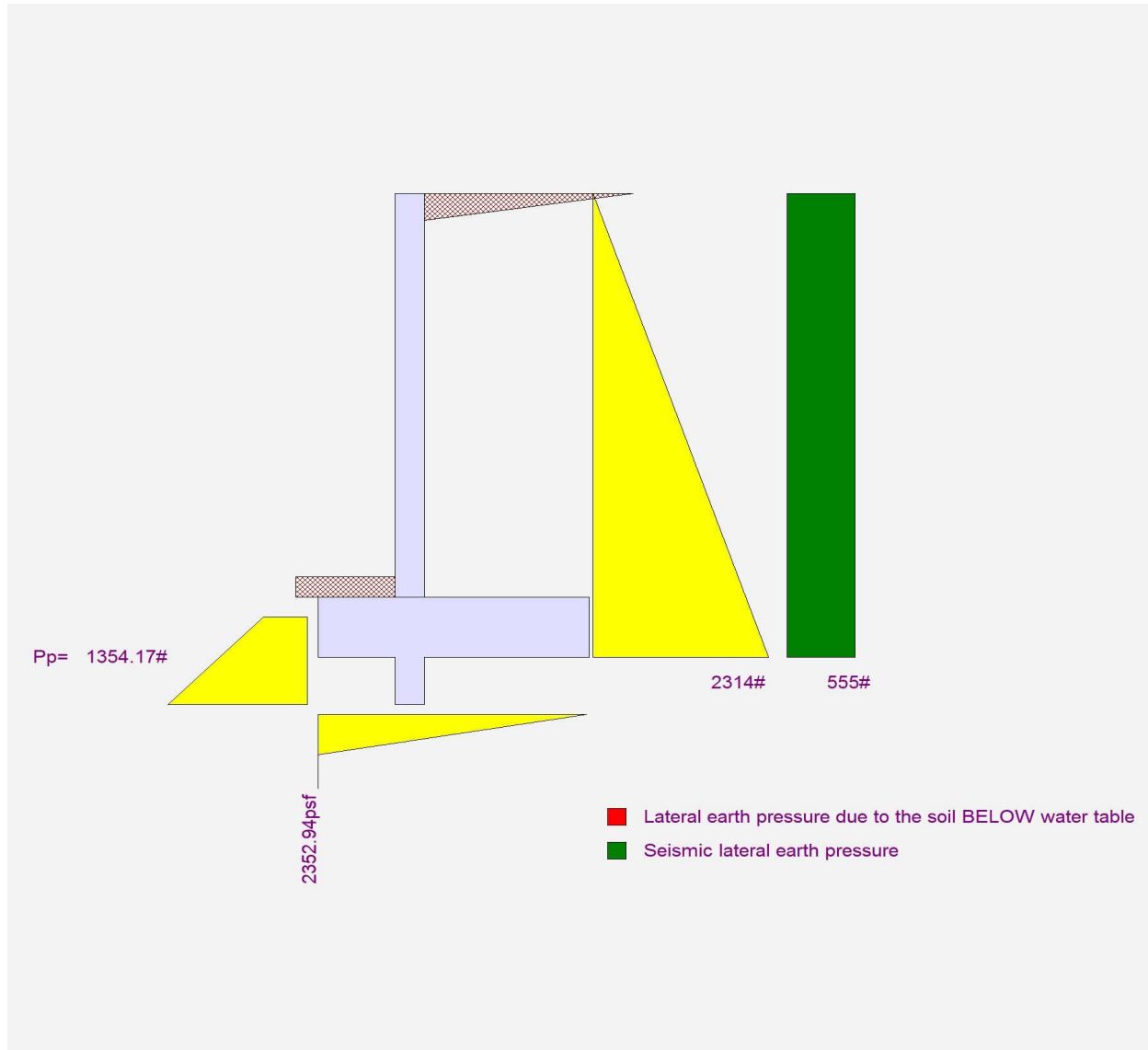
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 10'-0" Retaining Wall W/ Seismic



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall

Code Reference:

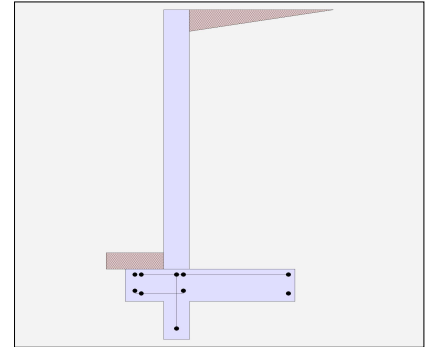
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall

Design Summary

Wall Stability Ratios

Overturning	=	2.60	OK
Sliding	=	1.56	OK
Global Stability	=	2.00	
Total Bearing Load	=	4,328 lbs	
...resultant ecc.	=	7.66 in	
Soil Pressure @ Toe	=	1,830 psf	OK
Soil Pressure @ Heel	=	130 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,562 psf	
ACI Factored @ Heel	=	182 psf	
Footing Shear @ Toe	=	6.3 psi	OK
Footing Shear @ Heel	=	11.0 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	1,417.5 lbs	
less 100% Passive Force	=	- 916.7 lbs	
less 100% Friction Force	=	- 1,298.5 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.588

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,792.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	4,778.7

Moment.....Allowable = 8,121.3

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	24.1

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 100.0

Rebar Depth 'd' in = 6.19

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1809 in2/ft		
(4/3) * As :	0.2413 in2/ft	Min Stem T&S Reinf Area 1.536 in2	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.2413 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.31 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.00 ft
Heel Width	=	3.42
Total Footing Width	=	4.42
Footing Thickness	=	12.00 in
Key Width	=	8.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm= 3.00 in

Footing Design Results

		Toe	Heel
Factored Pressure	=	2,562	182 psf
Mu' : Upward	=	1,191	2,555 ft-#
Mu' : Downward	=	128	5,216 ft-#
Mu: Design	=	1,064	2,661 ft-#
phiMn	=	11,610	13,005 ft-#
Actual 1-Way Shear	=	6.30	11.02 psi
Allow 1-Way Shear	=	75.00	75.00 psi
Toe Reinforcing	=	# 5 @ 12.00 in	
Heel Reinforcing	=	# 5 @ 12.00 in	
Key Reinforcing	=	# 4 @ 13.89 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	1.14	in2
Min footing T&S reinf Area per foot	0.26	in2 /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 9.26 in		#4@ 18.52 in
#5@ 14.35 in		#5@ 28.70 in
#6@ 20.37 in		#6@ 40.74 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	1,417.5	3.00	4,252.5	Soil Over HL (ab. water tbl)	2,749.3	3.04	8,361.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.04	8,361.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.50	
				Surcharge Over Toe =			
				Stem Weight(s) =	800.0	1.33	1,066.7
				Earth @ Stem Transitions =			
Total	= 1,417.5	O.T.M.	= 4,252.5	Footing Weight =	662.4	2.21	1,462.6
				Key Weight =	116.7	1.33	155.6
				Vert. Component =			
Resisting/Overturning Ratio		=	2.60	Total =	4,328.4 lbs	R.M.=	11,046.4
Vertical Loads used for Soil Pressure =		4,328.4 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.092 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #5 bar specified in this stem design segment = 23.40 in

Development length for #5 bar specified in this stem design segment = 18.00 in

Hooked embedment length into footing for #5 bar specified in this stem design segment = 8.17 in

As Provided = 0.3100 in/ft

As Required = 0.2413 in/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

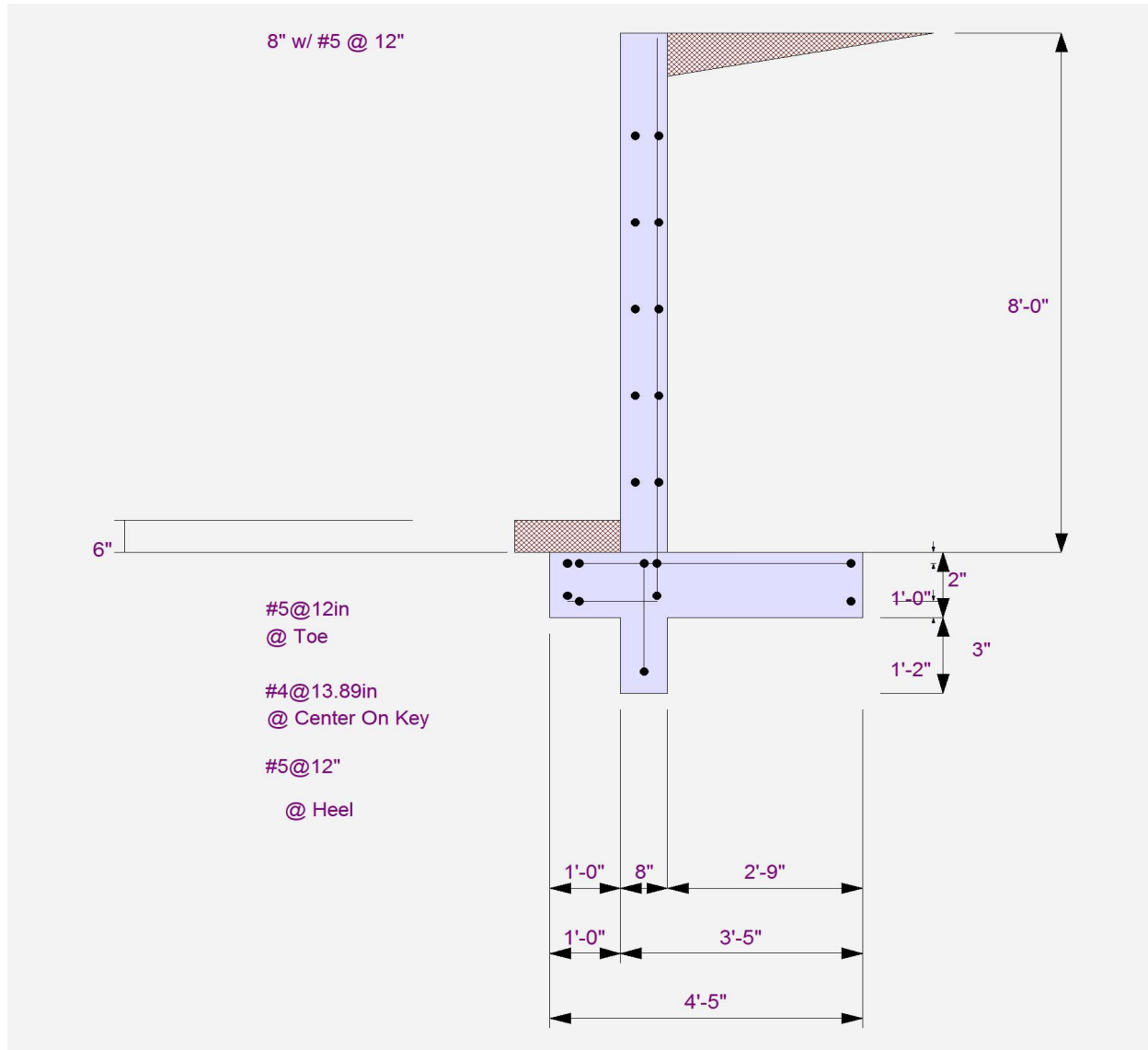
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

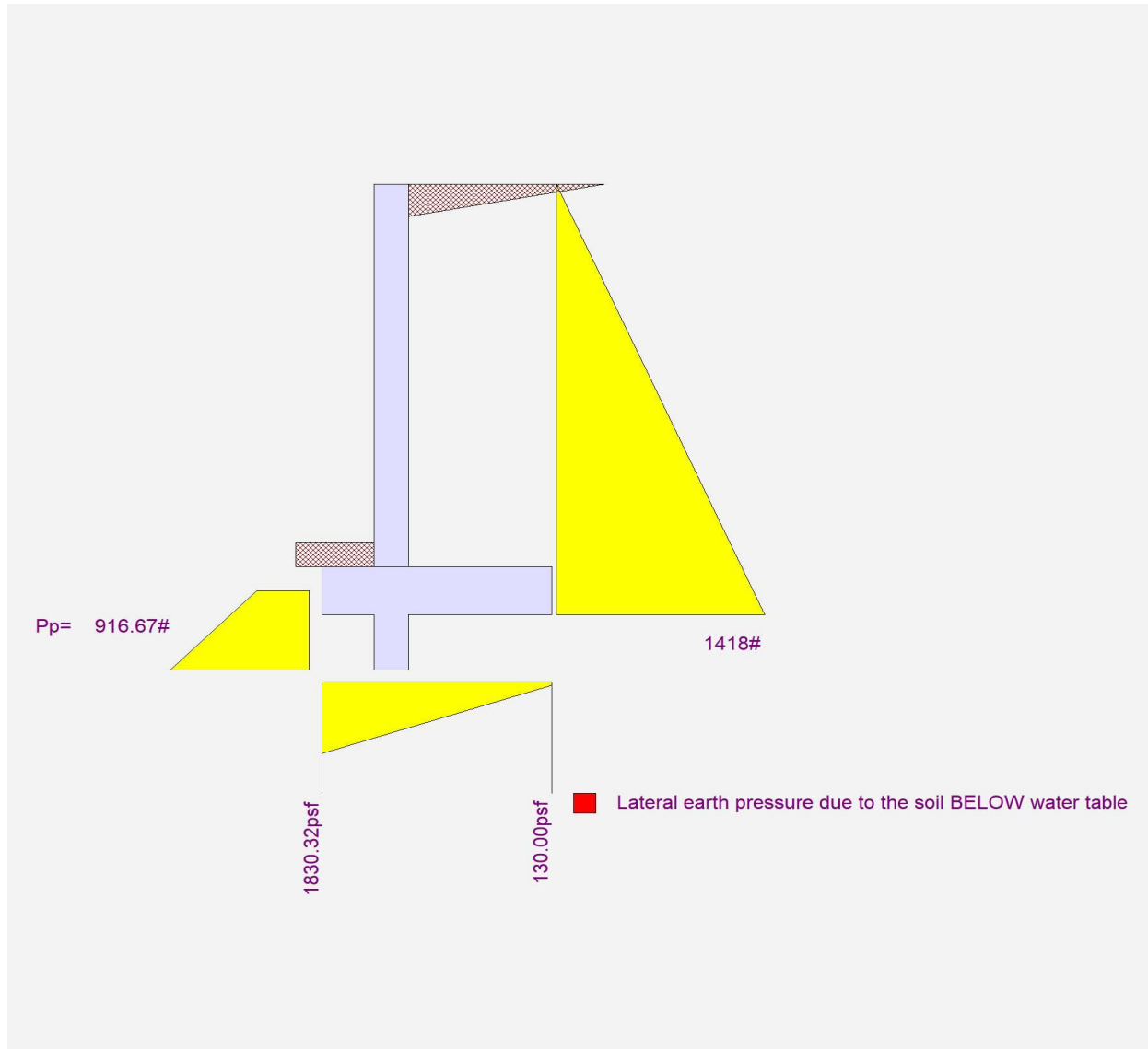
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic

Code Reference:

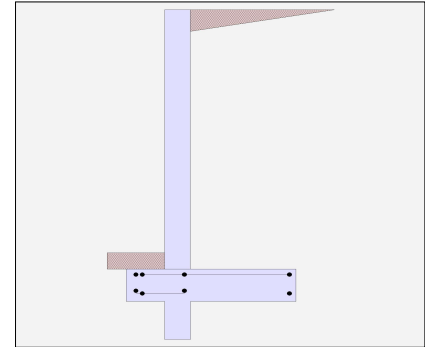
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	8.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	54.000
Total Seismic Force	=	486.000

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic

Design Summary

Wall Stability Ratios

Overturning	=	1.91	OK
Sliding	=	1.26	Ratio < 1.5!
Global Stability	=	2.00	
Total Bearing Load	=	4,328	lbs
...resultant ecc.	=	11.90	in
Soil Pressure @ Toe	=	2,373	psf OK
Soil Pressure @ Heel	=	0	psf OK
Allowable	=	4,000	psf
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	3,322	psf
ACI Factored @ Heel	=	0	psf
Footing Shear @ Toe	=	8.3	psi OK
Footing Shear @ Heel	=	17.6	psi OK
Allowable	=	75.0	psi

Sliding Calcs

Lateral Sliding Force	=	1,757.7	lbs
less 100% Passive Force	-	916.7	lbs
less 100% Friction Force	= -	1,298.5	lbs
Added Force Req'd	=	0.0	lbs OK
....for 1.5 Stability	=	421.4	lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 5
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.801
---------------	---	-------

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	2,224.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	6,506.7

Moment.....Allowable

=	8,121.3
---	---------

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	30.0

Shear.....Allowable

psi =	75.0
-------	------

Anet (Masonry)

in2 =	
-------	--

Wall Weight

psf =	100.0
-------	-------

Rebar Depth 'd'

in =	6.19
------	------

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.2464 in2/ft		
(4/3) * As :	0.3285 in2/ft	Min Stem T&S Reinf Area 1.536 in2	
200bd/fy : 200(12)(6.1875)/60000 :	0.2475 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.2475 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.31 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8382 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	1.00 ft
Heel Width	=	3.42
Total Footing Width	=	4.42
Footing Thickness	=	12.00 in
Key Width	=	8.00 in
Key Depth	=	14.00 in
Key Distance from Toe	=	1.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm = 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 3,322	0 psf
Mu' : Upward	= 1,509	1,180 ft-#
Mu' : Downward	= 128	5,216 ft-#
Mu: Design	= 1,382	4,035 ft-#
phiMn	= 11,610	13,005 ft-#
Actual 1-Way Shear	= 8.32	17.60 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 5 @ 12.00 in	
Heel Reinforcing	= # 5 @ 12.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	1.14	in2
Min footing T&S reinf Area per foot	0.26	in2 /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 9.26 in		#4@ 18.52 in
#5@ 14.35 in		#5@ 28.70 in
#6@ 20.37 in		#6@ 40.74 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	1,417.5	3.00	4,252.5	Soil Over HL (ab. water tbl)	2,749.3	3.04	8,361.6
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		3.04	8,361.6
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.50	
Seismic Earth Load =	340.2	4.50	1,530.9	Surcharge Over Toe =			
=				Stem Weight(s) =	800.0	1.33	1,066.7
Total =	1,757.7	O.T.M.	5,783.4	Earth @ Stem Transitions =			
				Footing Weight =	662.4	2.21	1,462.6
				Key Weight =	116.7	1.33	155.6
				Vert. Component =			
Resisting/Overturning Ratio		=	1.91	Total =	4,328.4 lbs	R.M.=	11,046.4
Vertical Loads used for Soil Pressure =		4,328.4 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.119 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #5 bar specified in this stem design segment = 23.40 in

Development length for #5 bar specified in this stem design segment = 18.00 in

Hooked embedment length into footing for #5 bar specified in this stem design segment = 8.38 in

As Provided = 0.3100 in²/ft

As Required = 0.2475 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

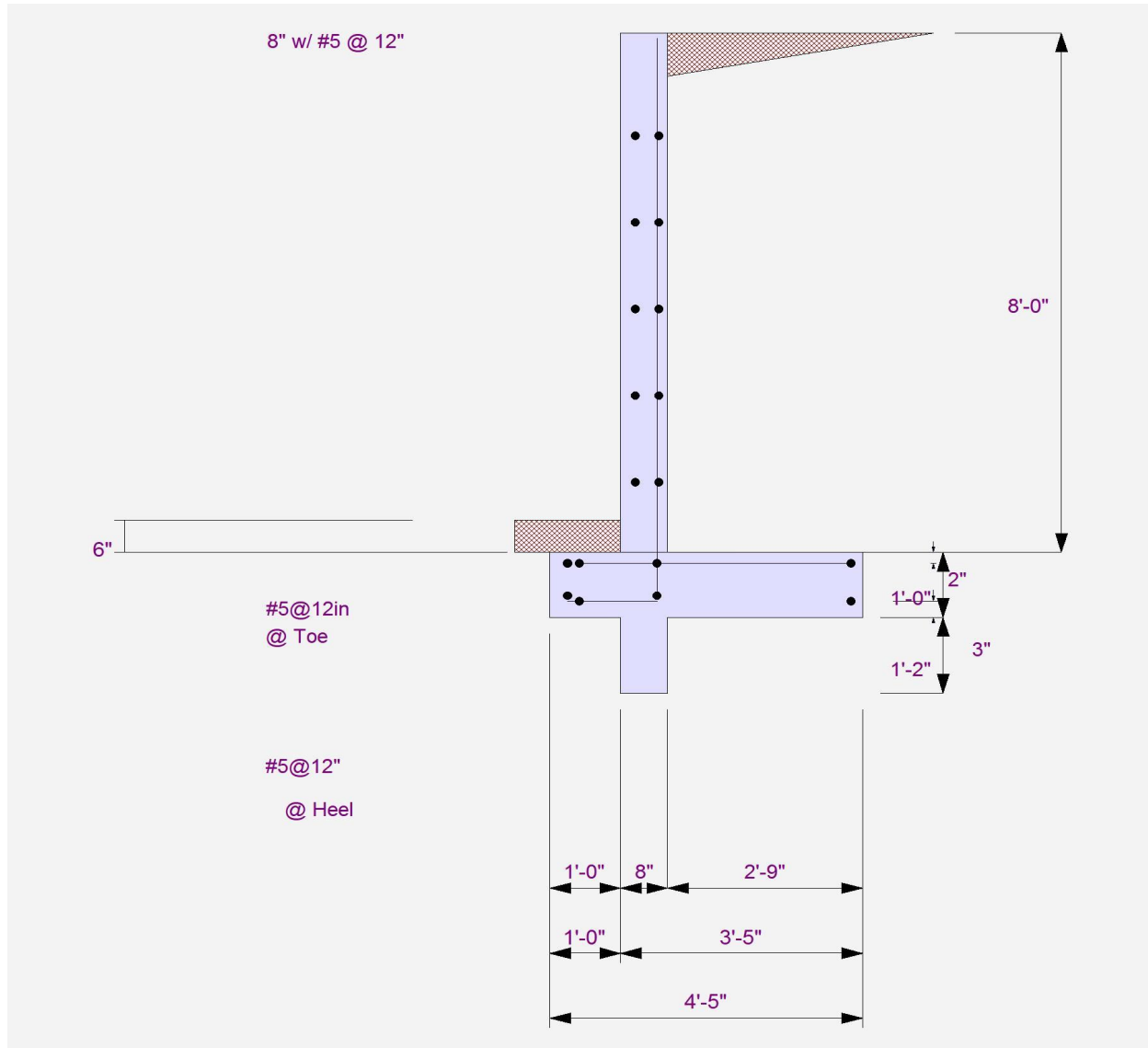
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

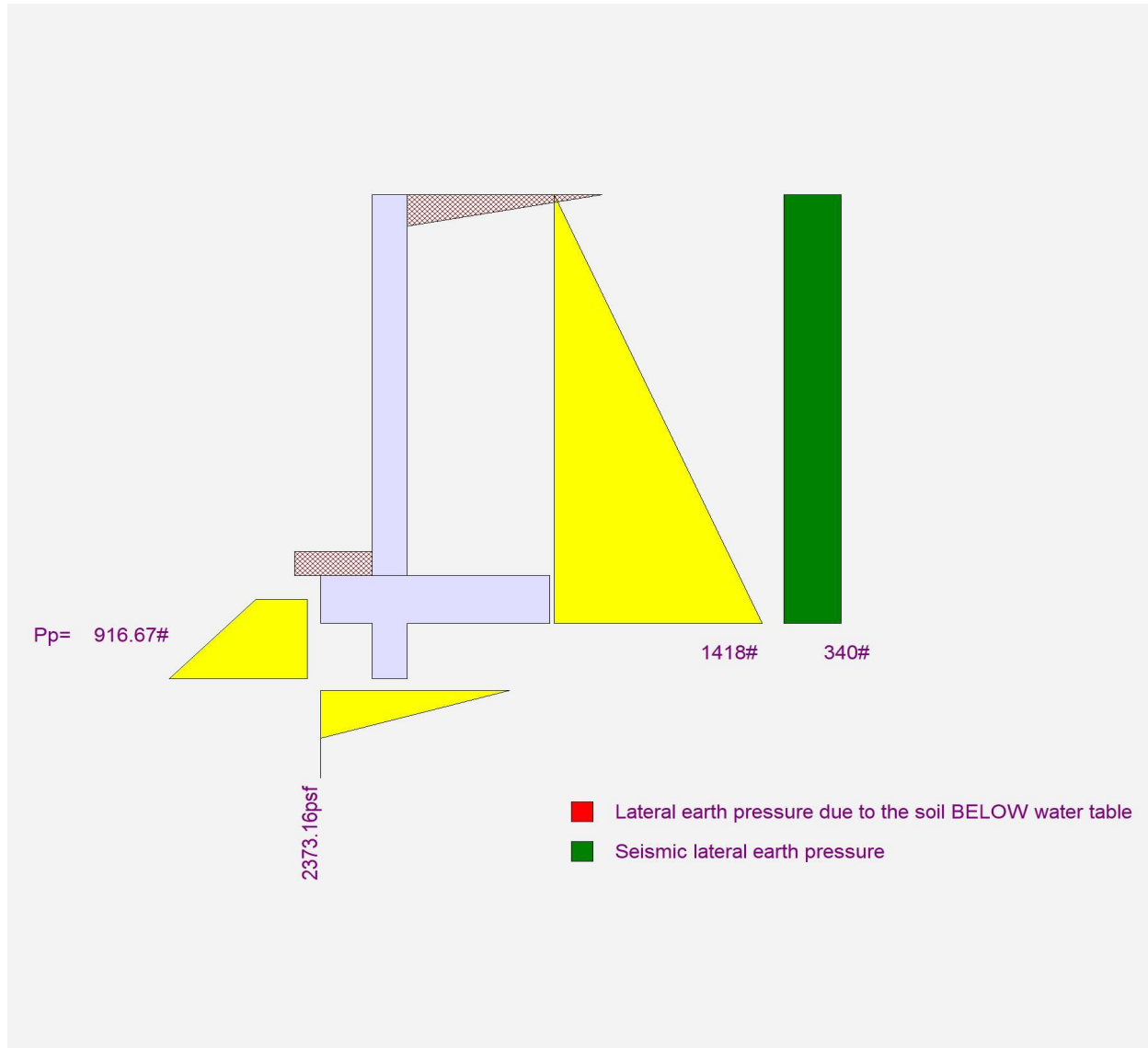
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 8'-0" Retaining Wall W/ Seismic



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall

Code Reference:

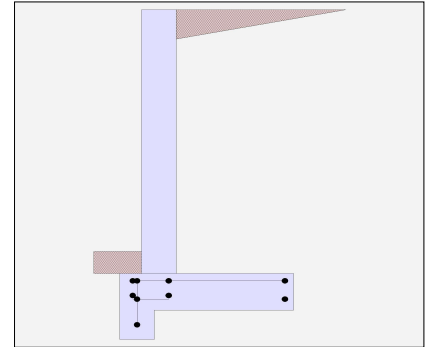
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall

Design Summary

Wall Stability Ratios

Overturning	=	2.64	OK
Sliding	=	1.57	OK
Global Stability	=	2.22	
Total Bearing Load	=	2,774 lbs	
...resultant ecc.	=	6.84 in	
Soil Pressure @ Toe	=	1,682 psf	OK
Soil Pressure @ Heel	=	0 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,355 psf	
ACI Factored @ Heel	=	0 psf	
Footing Shear @ Toe	=	0.4 psi	OK
Footing Shear @ Heel	=	6.9 psi	OK
Allowable	=	94.9 psi	

Sliding Calcs

Lateral Sliding Force	=	817.2 lbs	
less 100% Passive Force	= -	450.0 lbs	
less 100% Friction Force	= -	832.3 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS
 NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.367

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,008.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,016.0

Moment.....Allowable = 5,492.3

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	13.4

Shear.....Allowable psi = 94.9

Anet (Masonry)

Wall Weight psf = 100.0

Rebar Depth 'd' in = 6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	4,000.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0755 in2/ft		
(4/3) * As :	0.1007 in2/ft	Min Stem T&S Reinf Area 1.152 in2	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	1.3547 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.42 ft
Heel Width	=	2.92
Total Footing Width	=	3.34
Footing Thickness	=	10.00 in
Key Width	=	8.00 in
Key Depth	=	8.00 in
Key Distance from Toe	=	0.00 ft
f'c = 4,000 psi	Fy = 60,000 psi	
Footing Concrete Density = 150.00 pcf		
Min. As % = 0.0018		
Cover @ Top 2.00	@ Btm = 3.00 in	

Footing Design Results

		Toe	Heel
Factored Pressure	=	2,355	0 psf
Mu' : Upward	=	199	1,288 ft-#
Mu' : Downward	=	20	2,666 ft-#
Mu: Design	=	179	1,378 ft-#
phiMn	=	5,943	6,843 ft-#
Actual 1-Way Shear	=	0.39	6.88 psi
Allow 1-Way Shear	=	94.87	94.87 psi
Toe Reinforcing	=	# 4 @ 12.00 in	
Heel Reinforcing	=	# 4 @ 12.00 in	
Key Reinforcing	=	# 4 @ 13.89 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	0.72 in2	
Min footing T&S reinf Area per foot	0.22 in2 /ft	
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	817.2	2.28	1,861.3	Soil Over HL (ab. water tbl)	1,690.0	2.21	3,740.5
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.21	3,740.5
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.21	
				Surcharge Over Toe =			
				Stem Weight(s) =	600.0	0.75	452.0
				Earth @ Stem Transitions =			
Total	= 817.2	O.T.M. =	1,861.3	Footing Weight =	417.5	1.67	697.2
				Key Weight =	66.7	0.33	22.2
				Vert. Component =			
Resisting/Overturning Ratio		= 2.64		Total =	2,774.2 lbs	R.M.=	4,912.0
Vertical Loads used for Soil Pressure =		2,774.2 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.084 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment = 15.60 in

Development length for #4 bar specified in this stem design segment = 12.00 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 6.00 in

As Provided = 0.2000 in²/ft

As Required = 0.1728 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

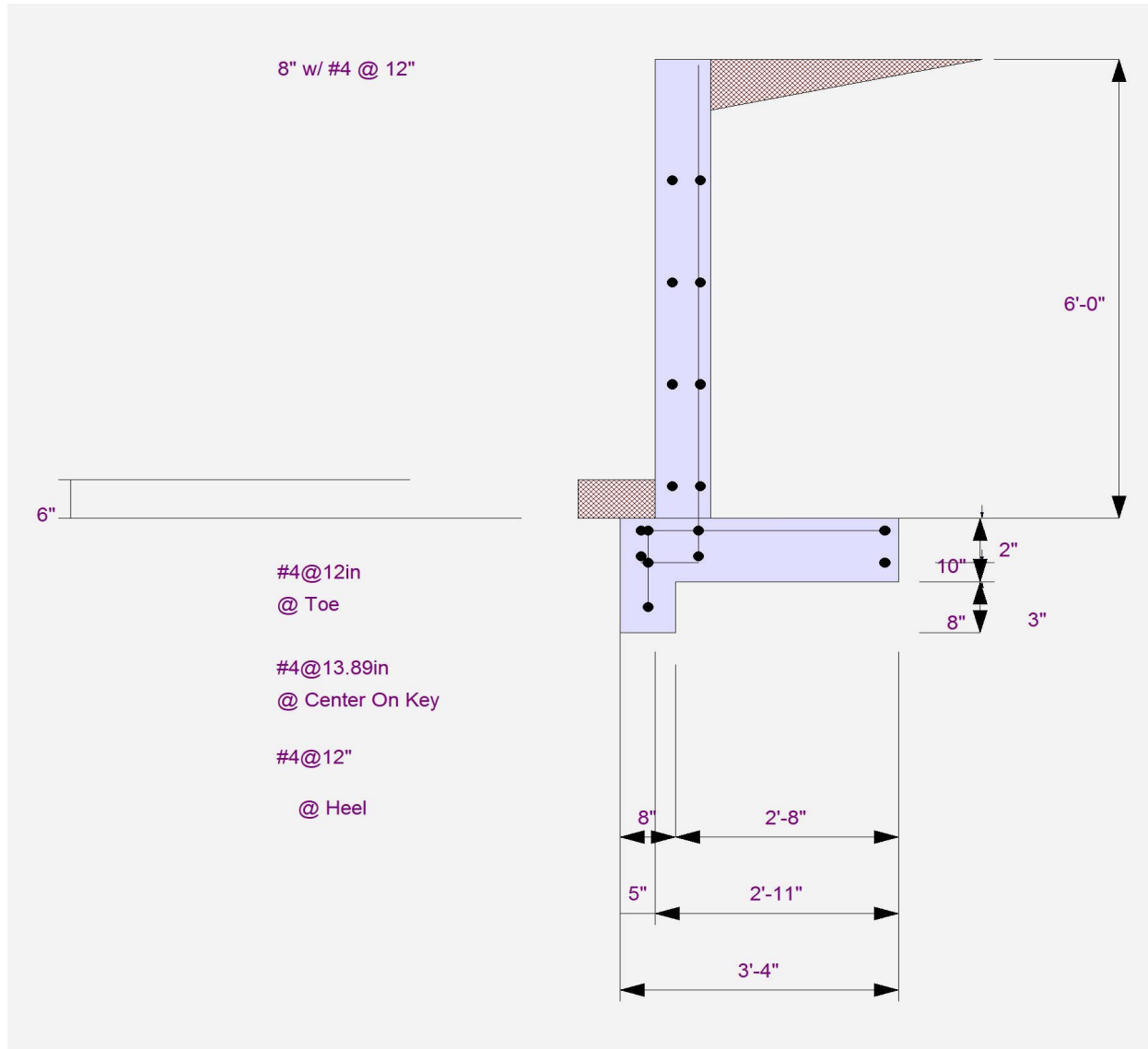
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

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DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

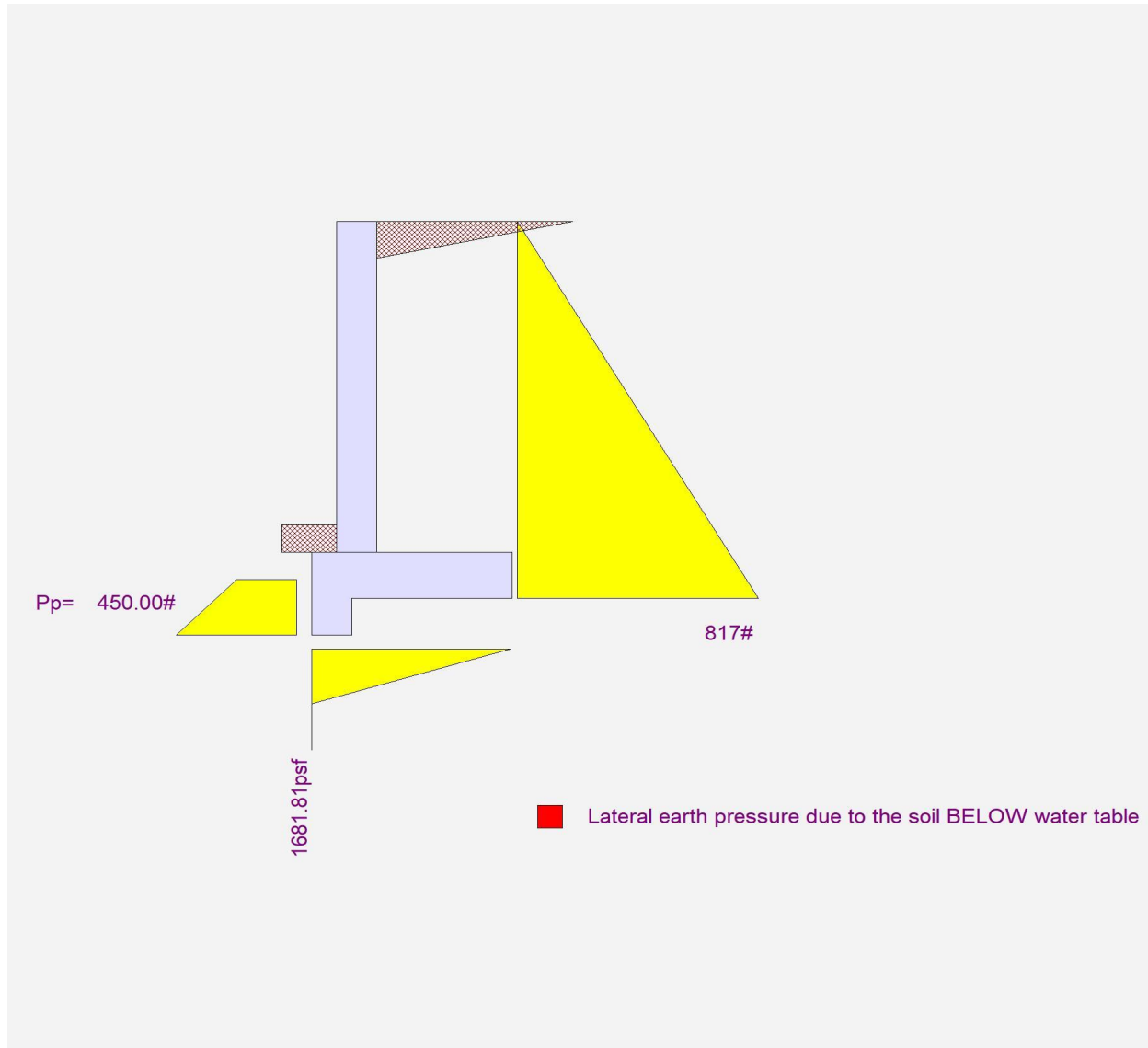
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic

Code Reference:

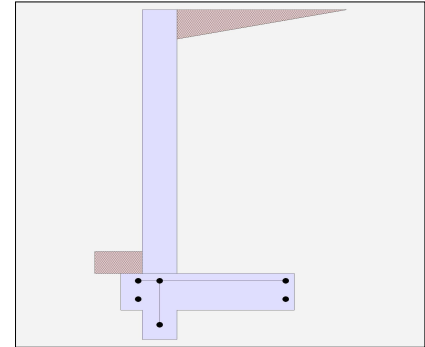
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	6.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	41.000
Total Seismic Force	=	280.167

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic

Design Summary

Wall Stability Ratios

Overturning	=	1.95	OK
Sliding	=	1.27	Ratio < 1.5!
Global Stability	=	2.22	
Total Bearing Load	=	2,774	lbs
...resultant ecc.	=	9.64	in
Soil Pressure @ Toe	=	2,139	psf OK
Soil Pressure @ Heel	=	0	psf OK
Allowable	=	4,000	psf
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,995	psf
ACI Factored @ Heel	=	0	psf
Footing Shear @ Toe	=	0.4	psi OK
Footing Shear @ Heel	=	11.6	psi OK
Allowable	=	75.0	psi

Sliding Calcs

Lateral Sliding Force	=	1,013.3	lbs
less 100% Passive Force	-	450.0	lbs
less 100% Friction Force	= -	832.1	lbs
Added Force Req'd	=	0.0	lbs OK
....for 1.5 Stability	=	237.8	lbs NG

Vertical component of active lateral soil pressure IS
 NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	12.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa	=	0.508
---------------	---	-------

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	1,254.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	2,754.0

Moment.....Allowable

=	5,412.6
---	---------

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	16.7

Shear.....Allowable

psi =	75.0
-------	------

Anet (Masonry)

in2 =	
-------	--

Wall Weight

psf =	100.0
-------	-------

Rebar Depth 'd'

in =	6.25
------	------

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.1032 in ² /ft		
(4/3) * As :	0.1376 in ² /ft	Min Stem T&S Reinf Area 1.152 in ²	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in ² /ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in ² /ft	
0.0018bh : 0.0018(12)(8) :	0.1728 in ² /ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1728 in ² /ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.2 in ² /ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in ² /ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.42 ft
Heel Width	=	<u>2.92</u>
Total Footing Width	=	3.34
Footing Thickness	=	10.00 in
Key Width	=	8.00 in
Key Depth	=	8.00 in
Key Distance from Toe	=	0.42 ft
f'c = 2,500 psi	Fy =	60,000 psi
Footing Concrete Density =		150.00 pcf
Min. As % =		0.0018
Cover @ Top 2.00	@ Btm=	3.00 in

Footing Design Results

		<u>Toe</u>	<u>Heel</u>
Factored Pressure	=	2,995	0 psf
Mu' : Upward	=	245	663 ft-#
Mu' : Downward	=	19	2,666 ft-#
Mu: Design	=	226	2,002 ft-#
phiMn	=	5,863	6,763 ft-#
Actual 1-Way Shear	=	0.40	11.65 psi
Allow 1-Way Shear	=	75.00	75.00 psi
Toe Reinforcing	=	# 4 @ 12.00 in	
Heel Reinforcing	=	# 4 @ 12.00 in	
Key Reinforcing	=	# 4 @ 13.89 in	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	0.72	in ²
Min footing T&S reinf Area per foot	0.22	in ² /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 11.11 in		#4@ 22.22 in
#5@ 17.22 in		#5@ 34.44 in
#6@ 24.44 in		#6@ 48.89 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	817.2	2.28	1,861.3	Soil Over HL (ab. water tbl)	1,690.0	2.21	3,733.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		2.21	3,733.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.21	
Seismic Earth Load =	196.1	3.42	670.1	Surcharge Over Toe =			
=				Stem Weight(s) =	600.0	0.75	449.6
Total =	1,013.3	O.T.M.	2,531.4	Earth @ Stem Transitions =			
				Footing Weight =	417.0	1.67	695.6
				Key Weight =	66.7	0.75	50.0
				Vert. Component =			
Resisting/Overturning Ratio		=	1.95	Total =	2,773.7 lbs	R.M.=	4,928.9
Vertical Loads used for Soil Pressure =		2,773.7 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.107 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment = 18.72 in

Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 7.26 in

As Provided = 0.2000 in²/ft

As Required = 0.1728 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

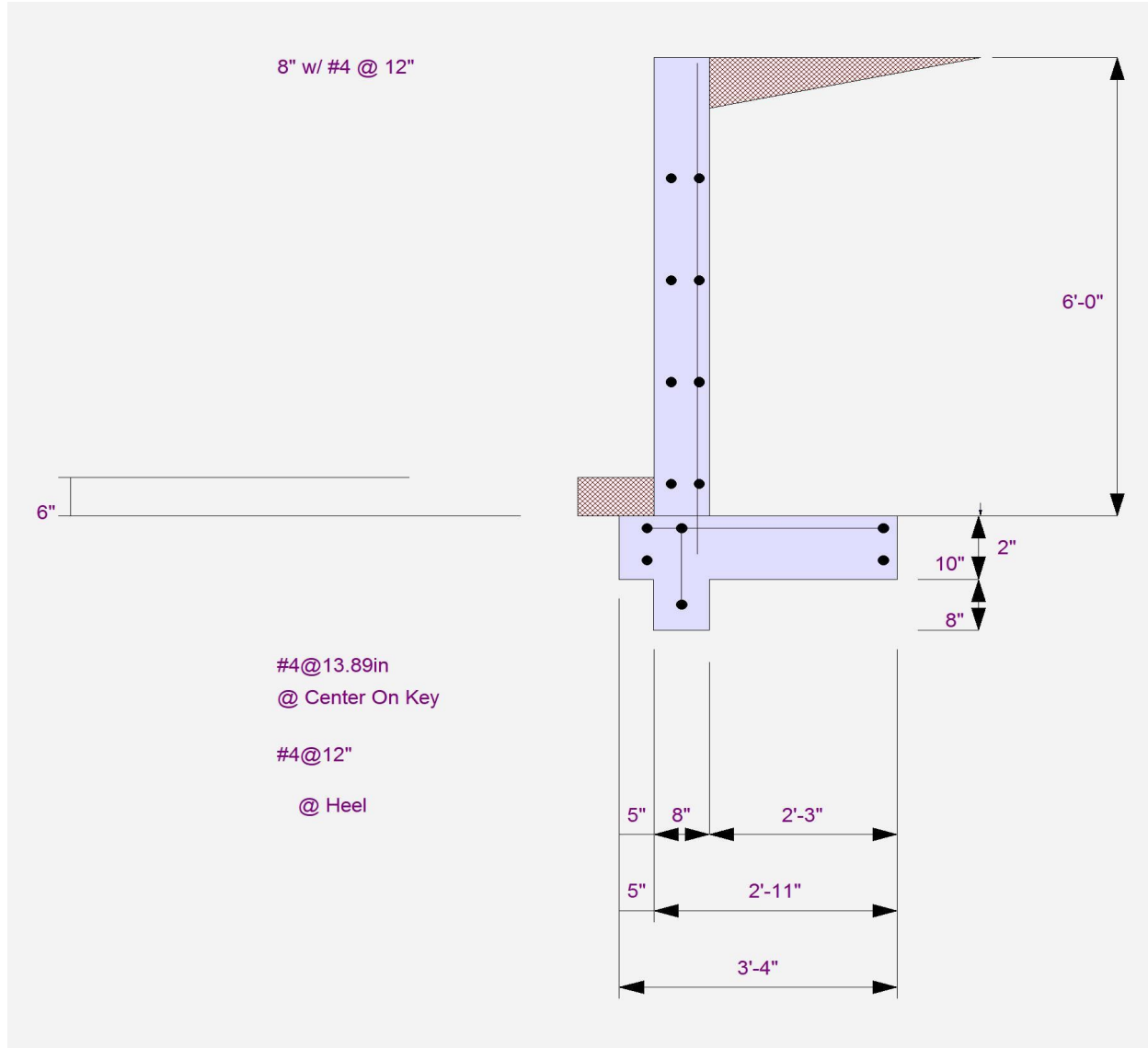
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

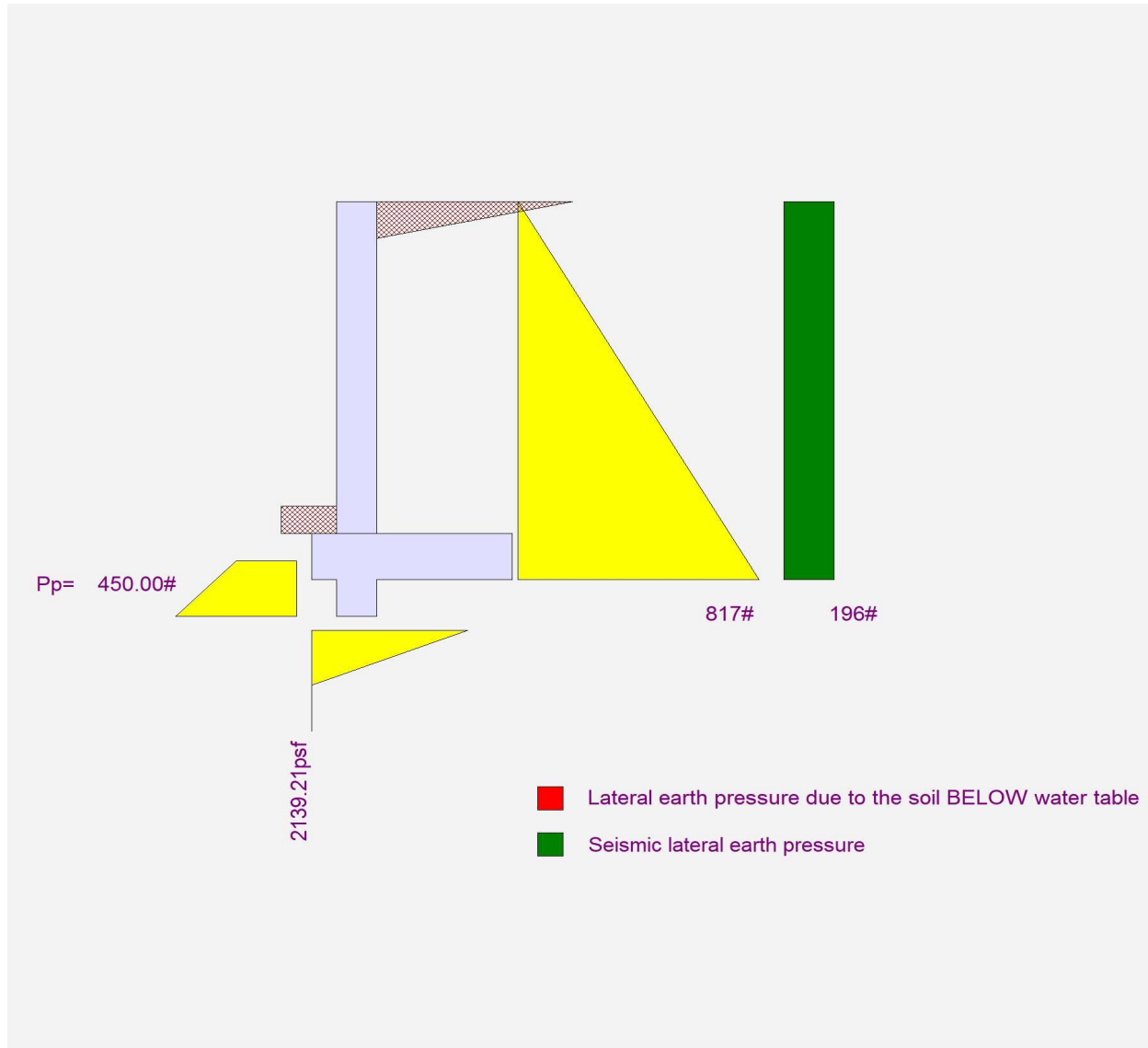
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 6'-0" Retaining Wall W/ Seismic



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall

Code Reference:

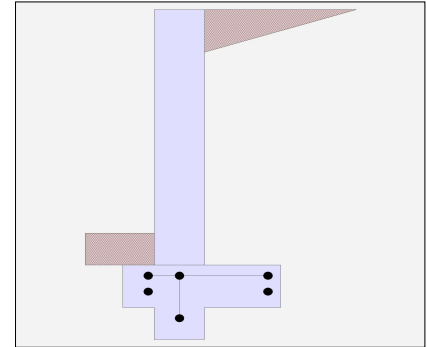
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	350.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall

Design Summary

Wall Stability Ratios

Overturning	=	2.28	OK
Sliding	=	1.73	OK
Global Stability	=	2.36	
Total Bearing Load	=	1,160 lbs	
...resultant ecc.	=	4.69 in	
Soil Pressure @ Toe	=	1,186 psf	OK
Soil Pressure @ Heel	=	0 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	1,660 psf	
ACI Factored @ Heel	=	0 psf	
Footing Shear @ Toe	=	1.1 psi	OK
Footing Shear @ Heel	=	6.0 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	381.1 lbs	
less 100% Passive Force	= -	311.1 lbs	
less 100% Friction Force	= -	348.1 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS
 NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	18.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.163

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	448.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	597.3

Moment.....Allowable = 3,655.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	6.0

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 100.0

Rebar Depth 'd' in = 6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0224 in2/ft		
(4/3) * As :	0.0298 in2/ft	Min Stem T&S Reinf Area 0.768 in2	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0012bh : 0.0012(12)(8) :	0.1152 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1152 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.1333 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.42 ft
Heel Width	=	1.67
Total Footing Width	=	2.09
Footing Thickness	=	8.00 in
Key Width	=	8.00 in
Key Depth	=	6.00 in
Key Distance from Toe	=	0.42 ft
f'c = 2,500 psi	Fy =	60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top 2.00	@ Btm.=	3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,660	0 psf
Mu' : Upward	= 133	94 ft-#
Mu' : Downward	= 17	362 ft-#
Mu: Design	= 117	268 ft-#
phiMn	= 4,063	4,963 ft-#
Actual 1-Way Shear	= 1.08	6.04 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 4 @ 12.00 in	
Heel Reinforcing	= # 4 @ 12.00 in	
Key Reinforcing	= # 4 @ 13.89 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	0.36 in2
Min footing T&S reinf Area per foot	0.17 in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 13.89 in	#4@ 27.78 in
#5@ 21.53 in	#5@ 43.06 in
#6@ 30.56 in	#6@ 61.11 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	381.1	1.56	592.8	Soil Over HL (ab. water tbl)	501.7	1.58	794.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		1.58	794.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.21	
				Surcharge Over Toe =			
				Stem Weight(s) =	400.0	0.75	299.7
				Earth @ Stem Transitions =			
Total	= 381.1	O.T.M.	= 592.8	Footing Weight =	208.6	1.04	217.6
				Key Weight =	50.0	0.75	37.5
				Vert. Component =			
Resisting/Overturning Ratio		=	2.28	Total =	1,160.3 lbs	R.M.=	1,349.6
Vertical Loads used for Soil Pressure =		1,160.3 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.063 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment = 18.72 in

Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 7.26 in

As Provided = 0.1333 in/ft

As Required = 0.1152 in/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

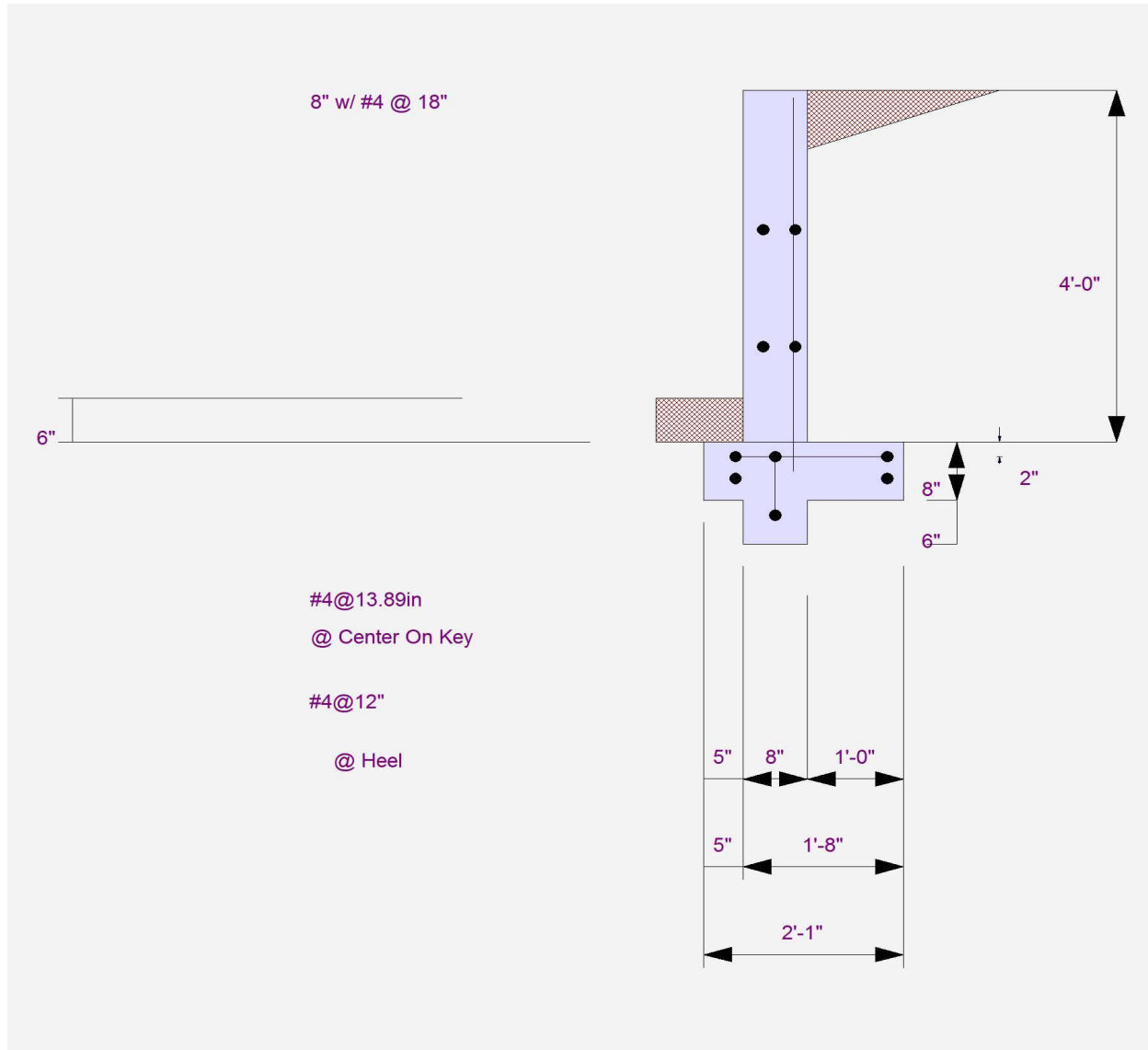
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

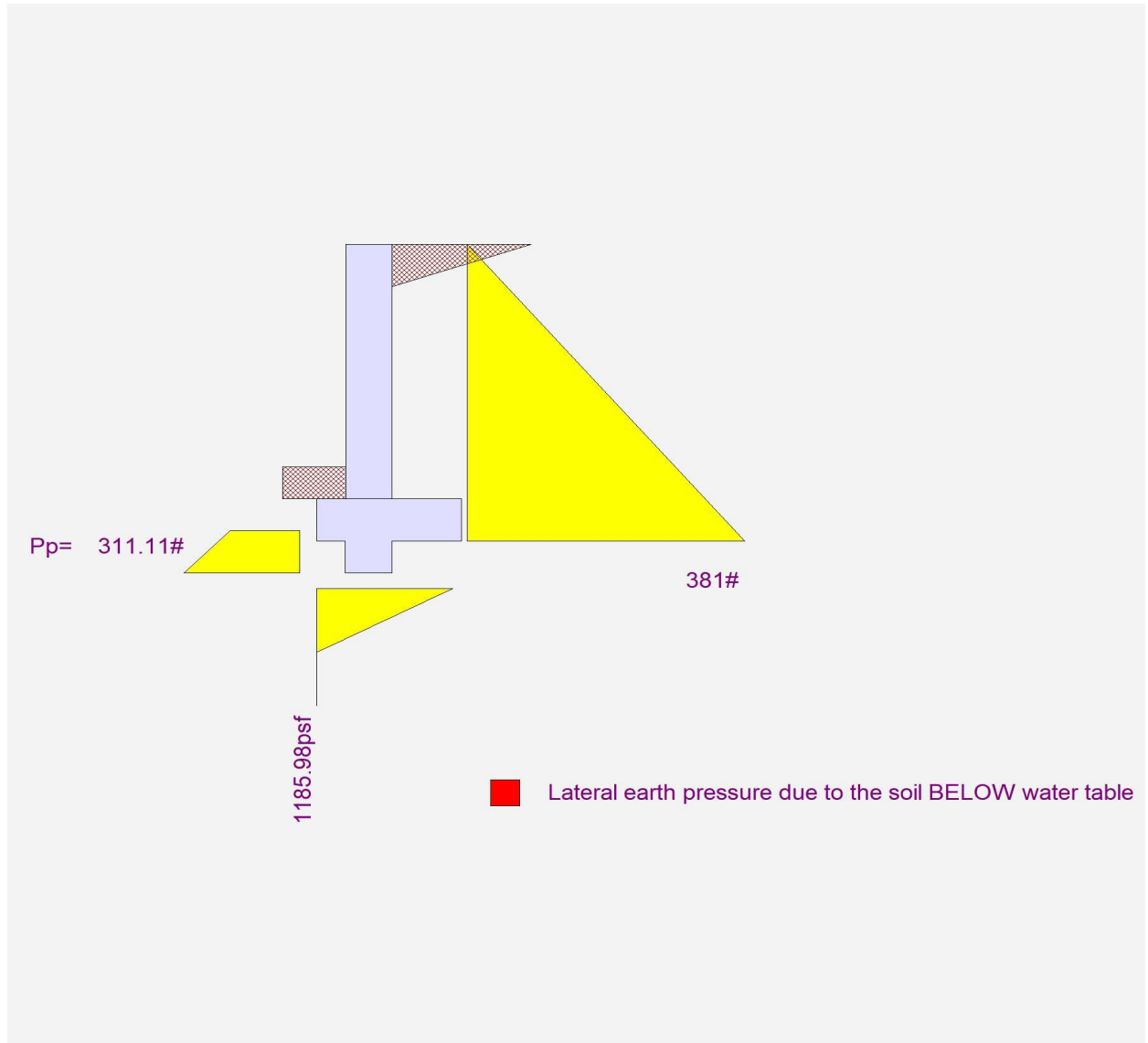
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic

Code Reference:

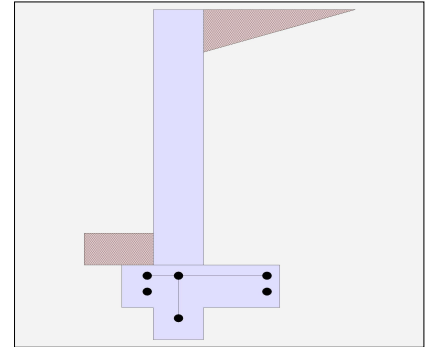
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	4.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	28.000
Total Seismic Force	=	130.667

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic

Design Summary

Wall Stability Ratios

Overturning	=	1.67	OK
Sliding	=	1.30	Ratio < 1.5!
Global Stability	=	2.36	
Total Bearing Load	=	1,160	lbs
...resultant ecc.	=	6.90	in
Soil Pressure @ Toe	=	1,652	psf OK
Soil Pressure @ Heel	=	0	psf OK
Allowable	=	4,000	psf
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,313	psf
ACI Factored @ Heel	=	0	psf
Footing Shear @ Toe	=	1.6	psi OK
Footing Shear @ Heel	=	9.7	psi OK
Allowable	=	75.0	psi

Sliding Calcs

Lateral Sliding Force	=	472.6	lbs
less 100% Passive Force	=	266.7	lbs
less 100% Friction Force	=	348.1	lbs
Added Force Req'd	=	0.0	lbs OK
....for 1.5 Stability	=	94.1	lbs NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	0.00
Wall Material Above "Ht"	= Concrete
Design Method	= SD
Thickness	= 8.00
Rebar Size	= # 4
Rebar Spacing	= 18.00
Rebar Placed at	= Edge

Design Data

fb/FB + fa/Fa = 0.224

Total Force @ Section

Service Level	lbs =
Strength Level	lbs = 560.0

Moment....Actual

Service Level	ft-# =
Strength Level	ft-# = 821.3

Moment.....Allowable = 3,655.6

Shear.....Actual

Service Level	psi =
Strength Level	psi = 7.5

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 100.0

Rebar Depth 'd' in = 6.25

Masonry Data

f'm	psi =
Fs	psi =
Solid Grouting	=
Modular Ratio 'n'	=
Equiv. Solid Thick.	=
Masonry Block Type	=
Masonry Design Method	= ASD

Concrete Data

f'c	psi = 2,500.0
Fy	psi = 60,000.0

Bottom

Stem OK

SD SD SD SD SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0308 in2/ft		
(4/3) * As :	0.041 in2/ft	Min Stem T&S Reinf Area 0.768 in2	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0012bh : 0.0012(12)(8) :	0.1152 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1152 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.1333 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.42 ft
Heel Width	=	1.67
Total Footing Width	=	2.09
Footing Thickness	=	8.00 in
Key Width	=	8.00 in
Key Depth	=	6.00 in
Key Distance from Toe	=	0.42 ft
f'c = 2,500 psi	Fy = 60,000 psi	
Footing Concrete Density = 150.00 pcf		
Min. As % = 0.0018		
Cover @ Top 2.00	@ Btm = 3.00 in	

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,313	0 psf
Mu' : Upward	= 180	9 ft-#
Mu' : Downward	= 17	362 ft-#
Mu: Design	= 163	353 ft-#
phiMn	= 4,063	4,963 ft-#
Actual 1-Way Shear	= 1.56	9.65 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 4 @ 12.00 in	
Heel Reinforcing	= # 4 @ 12.00 in	
Key Reinforcing	= # 4 @ 13.89 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	0.36 in2
Min footing T&S reinf Area per foot	0.17 in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 13.89 in	#4@ 27.78 in
#5@ 21.53 in	#5@ 43.06 in
#6@ 30.56 in	#6@ 61.11 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	381.1	1.56	592.8	Soil Over HL (ab. water tbl)	501.7	1.58	794.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		1.58	794.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.21	
Seismic Earth Load =	91.5	2.33	213.4	Surcharge Over Toe =			
=				Stem Weight(s) =	400.0	0.75	299.7
Total =	472.6	O.T.M.	806.3	Earth @ Stem Transitions =			
				Footing Weight =	208.6	1.04	217.6
				Key Weight =	50.0	0.75	37.5
				Vert. Component =			
				Total =	1,160.3 lbs	R.M.=	1,349.6

Resisting/Overturning Ratio = **1.67**
 Vertical Loads used for Soil Pressure = 1,160.3 lbs

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.088 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment = 18.72 in

Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 7.26 in

As Provided = 0.1333 in²/ft

As Required = 0.1152 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

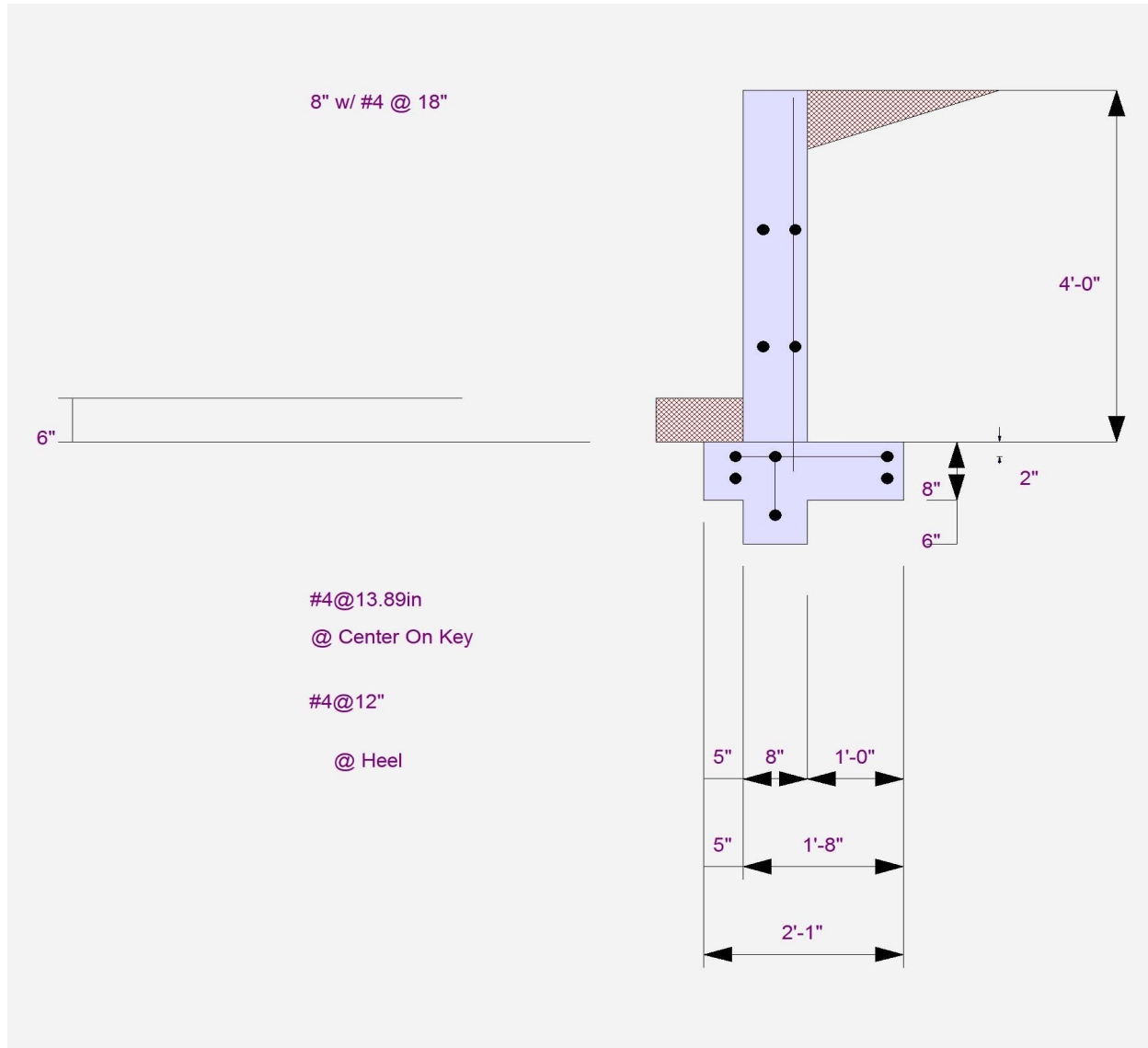
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

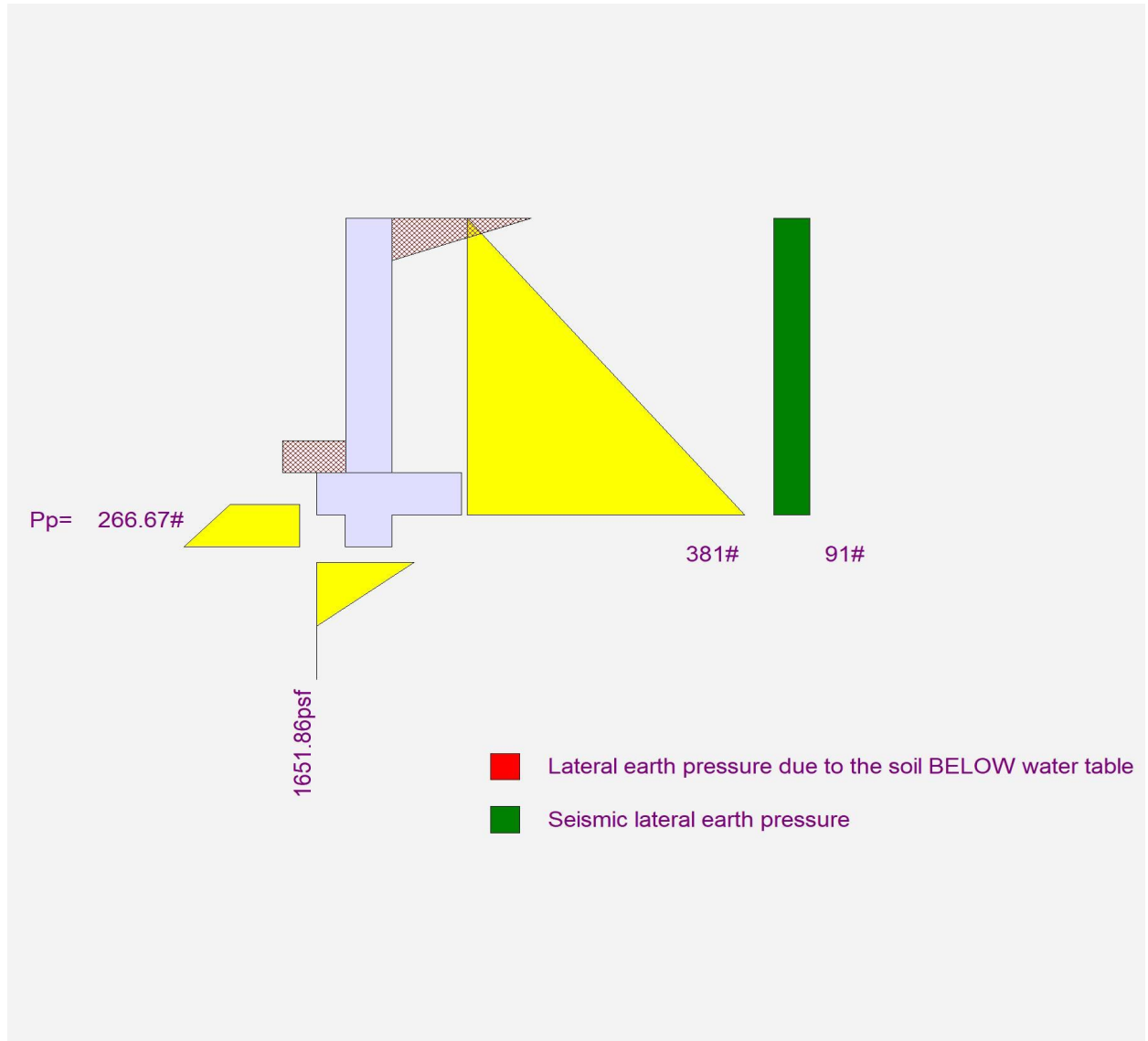
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 4'-0" Retaining Wall W/ Seismic



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall

Code Reference:

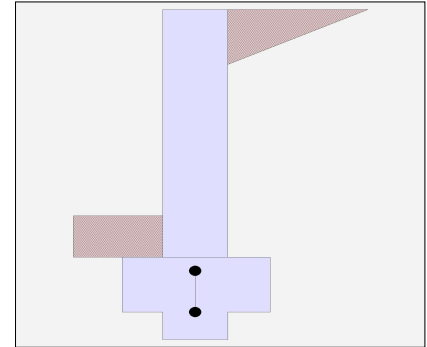
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	3.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall

Design Summary

Wall Stability Ratios

Overturing	=	2.00	OK
Sliding	=	1.62	OK
Global Stability	=	2.66	
Total Bearing Load	=	647 lbs	
...resultant ecc.	=	3.75 in	
Soil Pressure @ Toe	=	969 psf	OK
Soil Pressure @ Heel	=	0 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	1,357 psf	
ACI Factored @ Heel	=	0 psf	
Footing Shear @ Toe	=	0.6 psi	OK
Footing Shear @ Heel	=	3.0 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	235.3 lbs	
less 100% Passive Force	=	187.5 lbs	
less 100% Friction Force	=	194.2 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	0.0 lbs	OK

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	Stem OK	0.00
Wall Material Above "Ht"	=	Concrete
Design Method	=	SD
Thickness	=	8.00
Rebar Size	=	# 4
Rebar Spacing	=	18.00
Rebar Placed at	=	Edge

Design Data

fb/FB + fa/Fa = 0.068

Total Force @ Section

Service Level	lbs =	
Strength Level	lbs =	252.0

Moment....Actual

Service Level	ft-# =	
Strength Level	ft-# =	252.0

Moment.....Allowable = 3,655.6

Shear.....Actual

Service Level	psi =	
Strength Level	psi =	3.4

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 100.0

Rebar Depth 'd' in = 6.25

Masonry Data

f'm	psi =	
Fs	psi =	
Solid Grouting	=	
Modular Ratio 'n'	=	
Equiv. Solid Thick.	=	
Masonry Block Type	=	
Masonry Design Method	=	ASD

Concrete Data

f'c	psi =	2,500.0
Fy	psi =	60,000.0

Bottom

SD SD SD SD SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0094 in2/ft		
(4/3) * As :	0.0126 in2/ft	Min Stem T&S Reinf Area 0.576 in2	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0012bh : 0.0012(12)(8) :	0.1152 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1152 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.1333 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.42 ft
Heel Width	=	1.10
Total Footing Width	=	1.52
Footing Thickness	=	8.00 in
Key Width	=	8.00 in
Key Depth	=	4.00 in
Key Distance from Toe	=	0.42 ft
f'c = 2,500 psi	Fy =	60,000 psi
Footing Concrete Density =		150.00 pcf
Min. As % =		0.0018
Cover @ Top 2.00	@ Btm=	3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 1,357	0 psf
Mu' : Upward	= 105	3 ft-#
Mu' : Downward	= 17	54 ft-#
Mu: Design	= 88	51 ft-#
phiMn	= 900	900 ft-#
Actual 1-Way Shear	= 0.64	2.98 psi
Allow 1-Way Shear	= 40.00	40.00 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= # 4 @ 13.89 in	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	0.26 in2
Min footing T&S reinf Area per foot	0.17 in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 13.89 in	#4@ 27.78 in
#5@ 21.53 in	#5@ 43.06 in
#6@ 30.56 in	#6@ 61.11 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....		RESISTING.....			
	Force lbs	Distance ft	Moment ft-#	Force lbs	Distance ft	Moment ft-#	
HL Act Pres (ab water tbl)	235.3	1.22	287.6	Soil Over HL (ab. water tbl)	162.5	1.30	211.1
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		1.30	211.1
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.21	
				Surcharge Over Toe =			
				Stem Weight(s) =	300.0	0.75	224.8
				Earth @ Stem Transitions =			
Total	= 235.3	O.T.M.	= 287.6	Footing Weight =	151.6	0.76	114.9
				Key Weight =	33.3	0.75	25.0
				Vert. Component =			
Resisting/Overturning Ratio		=	2.00	Total =	647.4 lbs	R.M.=	575.8
Vertical Loads used for Soil Pressure =		647.4 lbs		* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.			

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.053 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment = 18.72 in

Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 7.26 in

As Provided = 0.1333 in²/ft

As Required = 0.1152 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

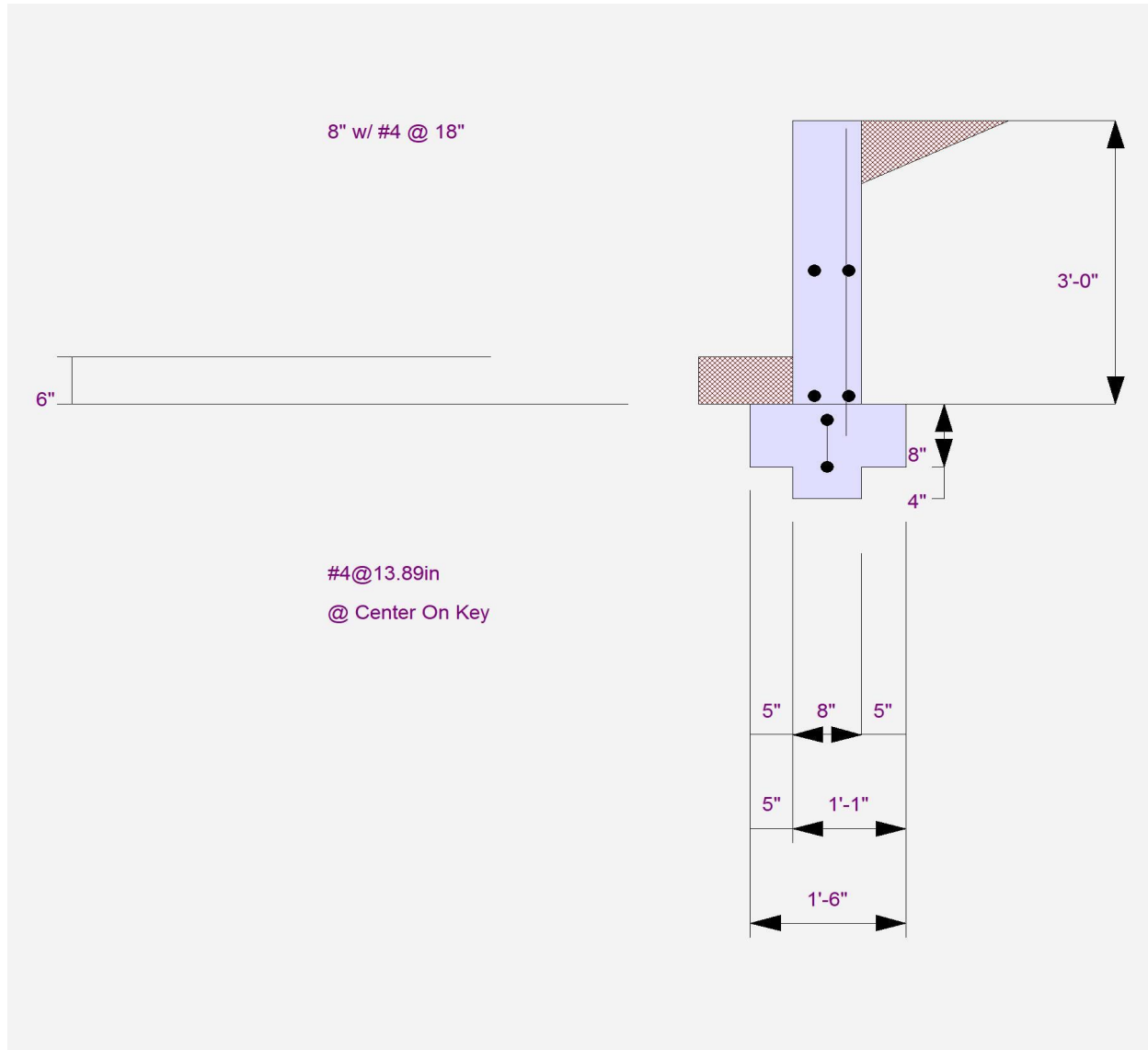
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

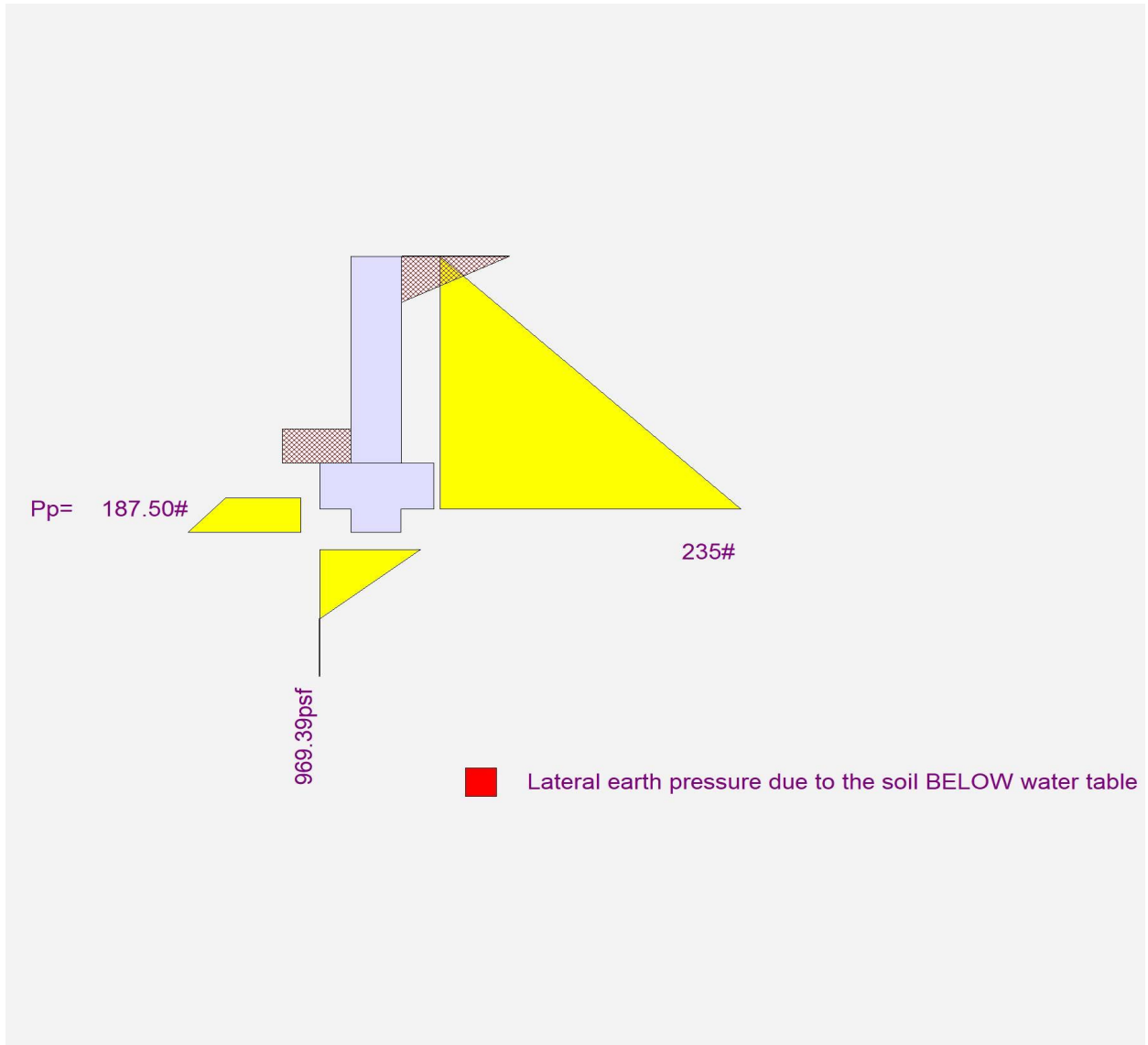
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic

Code Reference:

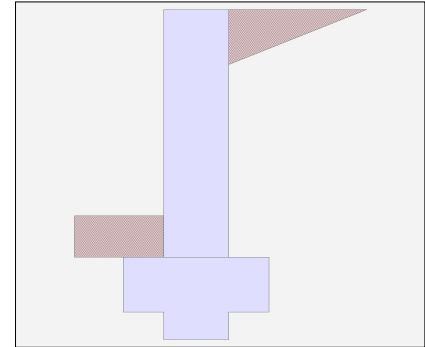
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	3.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.300
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	0.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Strength Level)

Uniform Seismic Force	=	22.000
Total Seismic Force	=	80.667

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic

Design Summary

Wall Stability Ratios

Overturning	=	1.44	Ratio < 1.5!
Sliding	=	1.30	Ratio < 1.5!
Global Stability	=	2.65	
Total Bearing Load	=	638 lbs	
...resultant ecc.	=	5.77 in	
Soil Pressure @ Toe	=	1,592 psf	OK
Soil Pressure @ Heel	=	0 psf	OK
Allowable	=	4,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,228 psf	
ACI Factored @ Heel	=	0 psf	
Footing Shear @ Toe	=	1.1 psi	OK
Footing Shear @ Heel	=	3.3 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	291.7 lbs	
less 100% Passive Force	= -	187.5 lbs	
less 100% Friction Force	= -	191.4 lbs	
Added Force Req'd	=	0.0 lbs	OK
....for 1.5 Stability	=	58.7 lbs	NG

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.600
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	0.00
Wall Material Above "Ht"	= Concrete
Design Method	= SD
Thickness	= 8.00
Rebar Size	= # 4
Rebar Spacing	= 18.00
Rebar Placed at	= Edge

Design Data

fb/FB + fa/Fa = 0.096

Total Force @ Section

Service Level	lbs =
Strength Level	lbs = 318.0

Moment....Actual

Service Level	ft-# =
Strength Level	ft-# = 351.0

Moment.....Allowable = 3,655.6

Shear.....Actual

Service Level	psi =
Strength Level	psi = 4.2

Shear.....Allowable psi = 75.0

Anet (Masonry) in2 =

Wall Weight psf = 100.0

Rebar Depth 'd' in = 6.25

Masonry Data

f'm	psi =
Fs	psi =
Solid Grouting	=
Modular Ratio 'n'	=
Equiv. Solid Thick.	=
Masonry Block Type	=
Masonry Design Method	= ASD

Concrete Data

f'c	psi = 2,500.0
Fy	psi = 60,000.0

Bottom

Stem OK					
Concrete					
SD	SD	SD	SD	SD	SD

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.0132 in2/ft		
(4/3) * As :	0.0175 in2/ft	Min Stem T&S Reinf Area 0.576 in2	
200bd/fy : 200(12)(6.25)/60000 :	0.25 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.192 in2/ft	
0.0012bh : 0.0012(12)(8) :	0.1152 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.1152 in2/ft	#4@ 12.50 in	#4@ 25.00 in
Provided Area :	0.1333 in2/ft	#5@ 19.38 in	#5@ 38.75 in
Maximum Area :	0.8467 in2/ft	#6@ 27.50 in	#6@ 55.00 in

Footing Data

Toe Width	=	0.42 ft
Heel Width	=	1.08
Total Footing Width	=	1.50
Footing Thickness	=	8.00 in
Key Width	=	8.00 in
Key Depth	=	4.00 in
Key Distance from Toe	=	0.42 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 2,228	0 psf
Mu' : Upward	= 159	0 ft-#
Mu' : Downward	= 17	49 ft-#
Mu: Design	= 143	49 ft-#
phiMn	= 900	900 ft-#
Actual 1-Way Shear	= 1.11	3.27 psi
Allow 1-Way Shear	= 40.00	40.00 psi
Toe Reinforcing	= None Spec'd	
Heel Reinforcing	= None Spec'd	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	0.26 in2
Min footing T&S reinf Area per foot	0.17 in2 /ft
If one layer of horizontal bars:	If two layers of horizontal bars:
#4@ 13.89 in	#4@ 27.78 in
#5@ 21.53 in	#5@ 43.06 in
#6@ 30.56 in	#6@ 61.11 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	235.3	1.22	287.6	Soil Over HL (ab. water tbl)	155.0	1.29	199.8
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		1.29	199.8
Hydrostatic Force				Watre Table			
Buoyant Force =				Sloped Soil Over Heel =			
Surcharge over Heel =				Surcharge Over Heel =			
Surcharge Over Toe =				Adjacent Footing Load =			
Adjacent Footing Load =				Axial Dead Load on Stem =			
Added Lateral Load =				* Axial Live Load on Stem =			
Load @ Stem Above Soil =				Soil Over Toe =		0.21	
Seismic Earth Load =	56.5	1.83	103.5	Surcharge Over Toe =			
=				Stem Weight(s) =	300.0	0.75	224.8
Total =	291.7	O.T.M.	391.1	Earth @ Stem Transitions =			
				Footing Weight =	149.6	0.75	111.9
				Key Weight =	33.3	0.75	25.0
				Vert. Component =			
Resisting/Overturning Ratio		=	1.44	Total =	637.9 lbs	R.M.=	561.5
Vertical Loads used for Soil Pressure =		637.9 lbs					

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.089 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #4 bar specified in this stem design segment = 18.72 in

Development length for #4 bar specified in this stem design segment = 14.40 in

Hooked embedment length into footing for #4 bar specified in this stem design segment = 7.26 in

As Provided = 0.1333 in²/ft

As Required = 0.1152 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

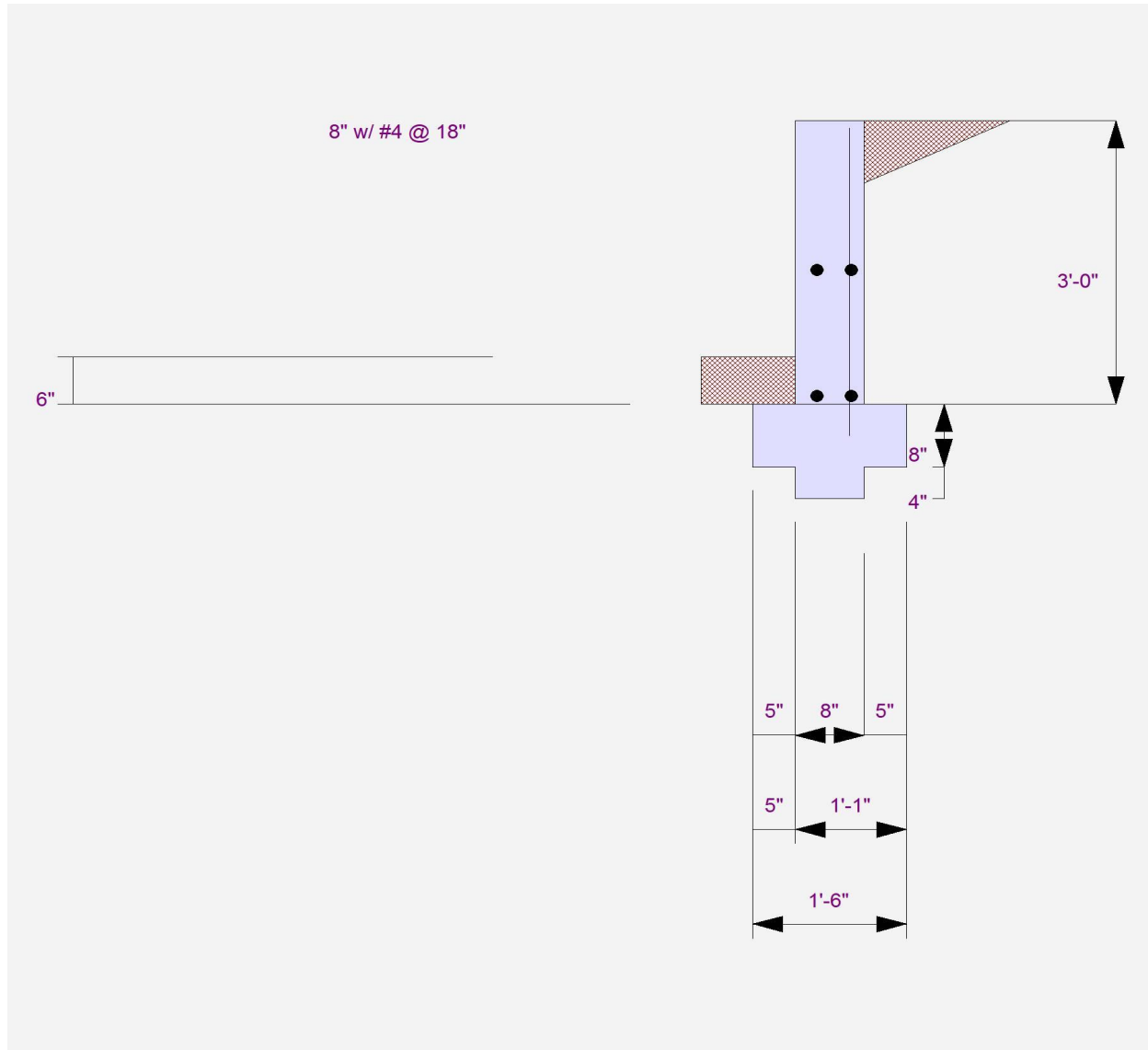
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic



Cantilevered Retaining Wall

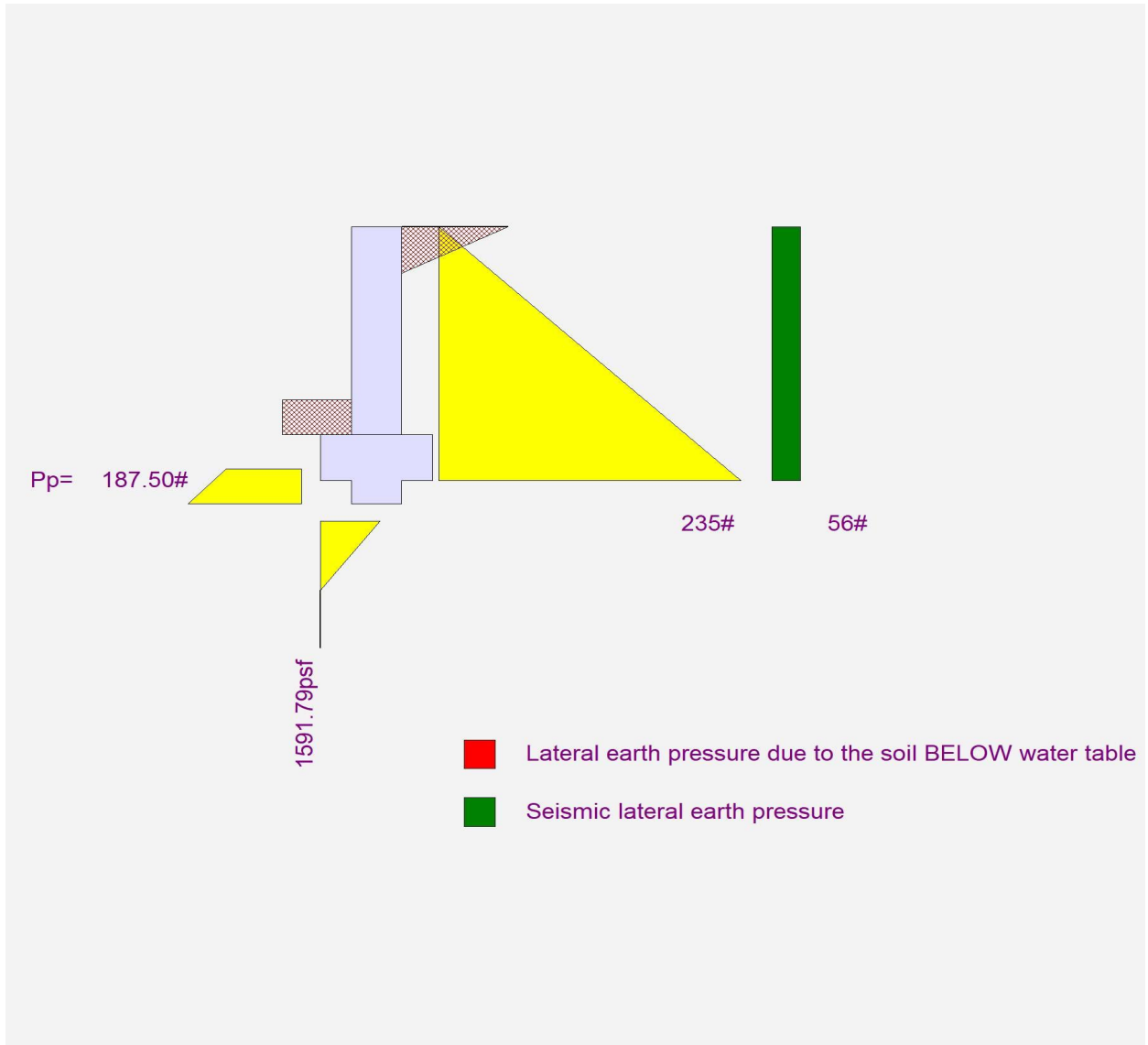
Project File: Typical Detail Co-04-06.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 3'-0" Retaining Wall W/Seismic



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic

Code Reference:

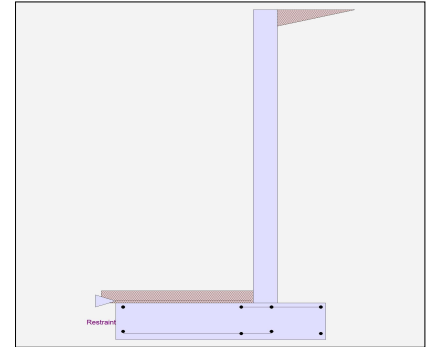
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	3,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	250.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic

Design Summary

Wall Stability Ratios

Overturning	=	1.56	OK
Slab Resists All Sliding !			
Global Stability	=	1.47	
Total Bearing Load	=	6,048 lbs	
...resultant ecc.	=	20.39 in	
Soil Pressure @ Toe	=	2,094 psf	OK
Soil Pressure @ Heel	=	0 psf	OK
Allowable	=	3,000 psf	
Soil Pressure Less Than Allowable			
ACI Factored @ Toe	=	2,931 psf	
ACI Factored @ Heel	=	0 psf	
Footing Shear @ Toe	=	33.8 psi	OK
Footing Shear @ Heel	=	22.1 psi	OK
Allowable	=	75.0 psi	

Sliding Calcs

Lateral Sliding Force	=	4,134.4 lbs
-----------------------	---	-------------

Vertical component of active lateral soil pressure IS NOT considered in the calculation of soil bearing

Load Factors

Building Code	
Dead Load	1.200
Live Load	1.600
Earth, H	1.600
Wind, W	1.000
Seismic, E	1.000

Stem Construction

Design Height Above Ftg

ft =	0.00
Wall Material Above "Ht"	= Concrete
Design Method	= SD
Thickness	= 10.00
Rebar Size	= # 7
Rebar Spacing	= 8.00
Rebar Placed at	= Edge

Design Data

fb/FB + fa/Fa	=	0.918
---------------	---	-------

Total Force @ Section

Service Level	lbs =
Strength Level	lbs = 5,376.0

Moment....Actual

Service Level	ft-# =
Strength Level	ft-# = 24,192.0

Moment.....Allowable

=	26,327.0
---	----------

Shear.....Actual

Service Level	psi =
Strength Level	psi = 59.2

Shear.....Allowable

psi =	75.0
-------	------

Anet (Masonry)

in2 =	
-------	--

Wall Weight

psf =	125.0
-------	-------

Rebar Depth 'd'

in =	7.56
------	------

Masonry Data

f'm	psi =
Fs	psi =
Solid Grouting	=
Modular Ratio 'n'	=
Equiv. Solid Thick.	=
Masonry Block Type	=
Masonry Design Method	= ASD

Concrete Data

f'c	psi = 2,500.0
Fy	psi = 60,000.0

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.7422 in2/ft		
(4/3) * As :	0.9895 in2/ft	Min Stem T&S Reinf Area 2.880 in2	
200bd/fy : 200(12)(7.5625)/60000 :	0.3025 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft	
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.7422 in2/ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.9 in2/ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0245 in2/ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	4.75 ft
Heel Width	=	2.50
Total Footing Width	=	7.25
Footing Thickness	=	18.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c = 2,500 psi	Fy =	60,000 psi
Footing Concrete Density =		150.00 pcf
Min. As % =		0.0018
Cover @ Top 2.00	@ Btm=	3.00 in

Footing Design Results

		Toe	Heel
Factored Pressure	=	2,931	0 psf
Mu' : Upward	=	24,004	1 ft-#
Mu' : Downward	=	3,892	3,431 ft-#
Mu: Design	=	20,112	3,430 ft-#
phiMn	=	54,690	29,913 ft-#
Actual 1-Way Shear	=	33.84	22.08 psi
Allow 1-Way Shear	=	75.00	75.00 psi
Toe Reinforcing	=	# 7 @ 8.00 in	
Heel Reinforcing	=	# 6 @ 12.00 in	
Key Reinforcing	=	None Spec'd	
Footing Torsion, Tu	=		0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=		0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	2.82	in2
Min footing T&S reinf Area per foot	0.39	in2 /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 6.17 in		#4@ 12.35 in
#5@ 9.57 in		#5@ 19.14 in
#6@ 13.58 in		#6@ 27.16 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	3,189.4	4.50	14,352.2	Soil Over HL (ab. water tbl)	2,500.0	6.42	16,041.7
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		6.42	16,041.7
Hydrostatic Force				Watre Table			
Buoyant Force	=			Sloped Soil Over Heel	=		
Surcharge over Heel	=	945.0	6,378.8	Surcharge Over Heel	=	416.7	2,673.6
Surcharge Over Toe	=			Adjacent Footing Load	=		
Adjacent Footing Load	=			Axial Dead Load on Stem	=		
Added Lateral Load	=			* Axial Live Load on Stem	=		
Load @ Stem Above Soil	=			Soil Over Toe	=	2.38	
	=			Surcharge Over Toe	=		
Total	=	4,134.4	O.T.M. = 20,730.9	Stem Weight(s)	=	1,500.0	7,750.0
				Earth @ Stem Transitions	=		
Resisting/Overturning Ratio		=	1.56	Footing Weight	=	1,631.3	5,913.3
Vertical Loads used for Soil Pressure =		6,047.9	lbs	Key Weight	=		
				Vert. Component	=		
				Total =	6,047.9	lbs	R.M.= 32,378.6

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci
 Horizontal Defl @ Top of Wall (approximate only) 0.096 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #7 bar specified in this stem design segment = 40.95 in

Development length for #7 bar specified in this stem design segment = 31.50 in

Hooked embedment length into footing for #7 bar specified in this stem design segment = 12.12 in

As Provided = 0.9000 in²/ft

As Required = 0.7422 in²/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

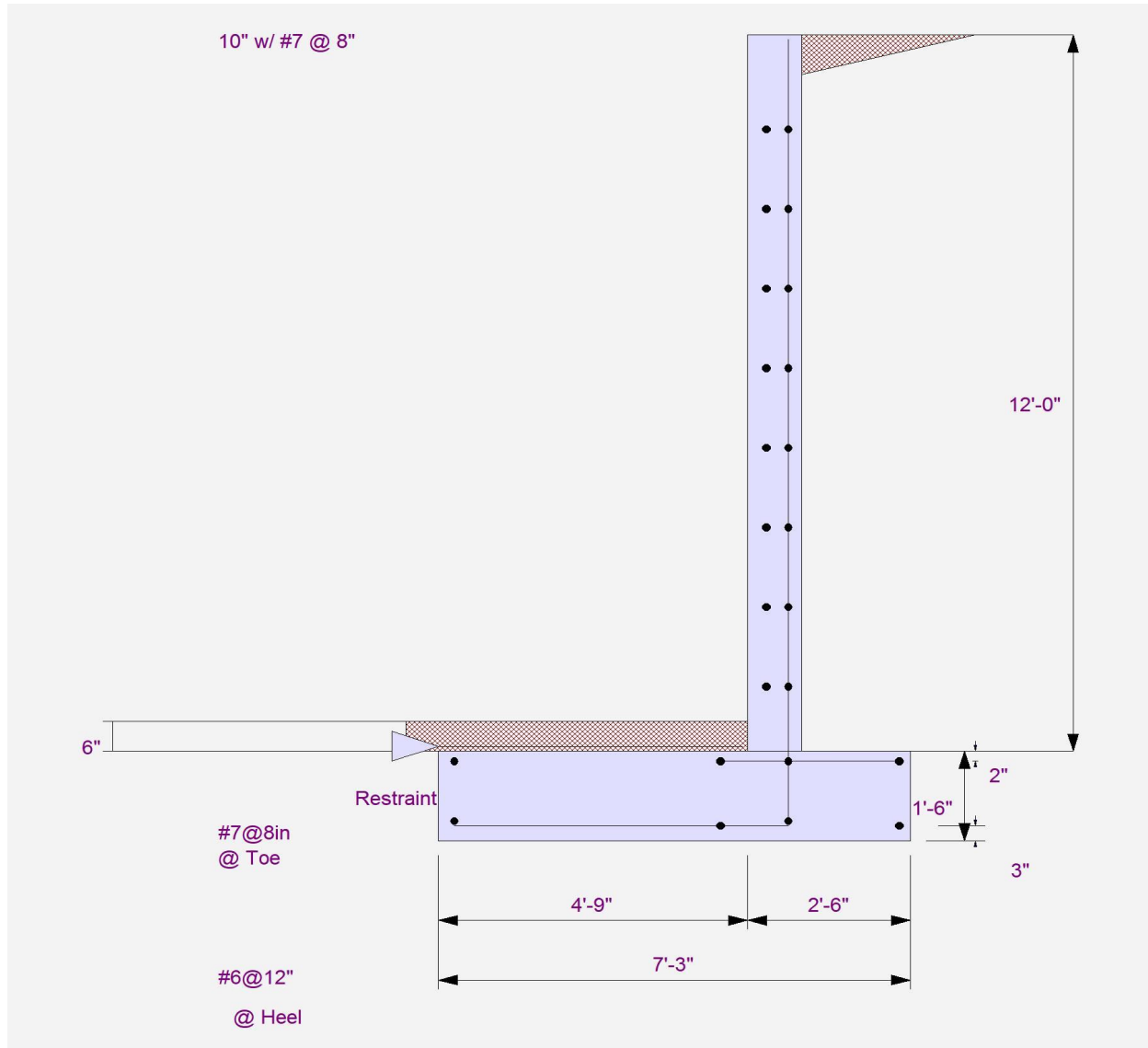
Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic



Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

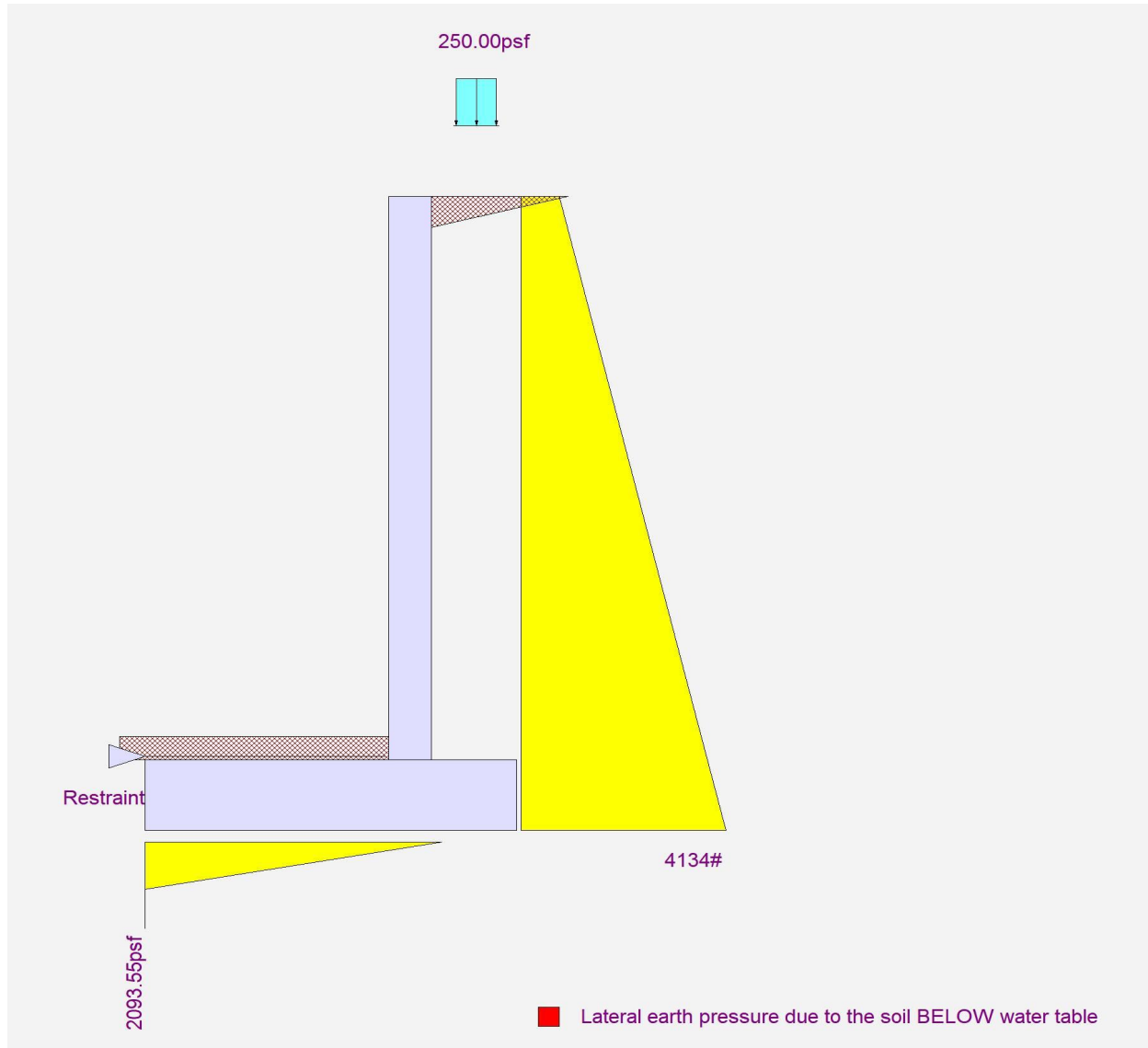
Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic



Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic

Code Reference

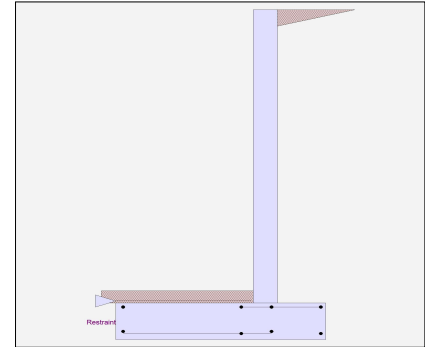
Calculations per IBC 2018 1807.3, CBC 2019, ASCE 7-16

Criteria

Retained Height	=	12.00 ft
Wall height above soil	=	0.00 ft
Slope Behind Wall	=	0.00
Height of Soil over Toe	=	6.00 in
Water height over heel	=	0.0 ft

Soil Data

Allow Soil Bearing	=	4,000.0 psf
Equivalent Fluid Pressure Method		
Active Heel Pressure	=	35.0 psf/ft
	=	
Passive Pressure	=	300.0 psf/ft
Soil Density, Heel	=	125.00 pcf
Soil Density, Toe	=	0.00 pcf
Footing Soil Friction	=	0.450
Soil height to ignore for passive pressure	=	12.00 in



Surcharge Loads

Surcharge Over Heel	=	250.0 psf
Used To Resist Sliding & Overturning		
Surcharge Over Toe	=	0.0
Used for Sliding & Overturning		

Axial Load Applied to Stem

Axial Dead Load	=	0.0 lbs
Axial Live Load	=	0.0 lbs
Axial Load Eccentricity	=	0.0 in

Earth Pressure Seismic Load

Method	:	Uniform
Multiplier Used	=	6.000
(Multiplier used on soil density)		

Lateral Load Applied to Stem

Lateral Load	=	0.0 #/ft
...Height to Top	=	0.00 ft
...Height to Bottom	=	0.00 ft
Load Type	=	Wind (W) (Service Level)
Wind on Exposed Stem	=	0.0 psf (Service Level)

Uniform Seismic Force	=	81.000
Total Seismic Force	=	1,093.500

Adjacent Footing Load

Adjacent Footing Load	=	0.0 lbs
Footing Width	=	0.00 ft
Eccentricity	=	0.00 in
Wall to Ftg CL Dist	=	0.00 ft
Footing Type	=	Spread Footing
Base Above/Below Soil at Back of Wall	=	0.0 ft
Poisson's Ratio	=	0.300

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic

Design Summary

Wall Stability Ratios

Overturning = 1.25 Ratio < 1.5!
 Slab Resists All Sliding !
 Global Stability = 1.47
 Total Bearing Load = 6,048 lbs
 ...resultant ecc. = 30.64 in
 Soil Pressure @ Toe = 3,763 psf OK
 Soil Pressure @ Heel = 0 psf OK
 Allowable = 4,000 psf
 Soil Pressure Less Than Allowable
 ACI Factored @ Toe = 5,268 psf
 ACI Factored @ Heel = 0 psf
 Footing Shear @ Toe = 41.1 psi OK
 Footing Shear @ Heel = 22.1 psi OK
 Allowable = 75.0 psi

Sliding Calcs

Lateral Sliding Force = 4,899.8 lbs

Vertical component of active lateral soil pressure IS
 NOT considered in the calculation of soil bearing

Load Factors

Building Code
 Dead Load 1.200
 Live Load 1.600
 Earth, H 1.600
 Wind, W 1.000
 Seismic, E 1.000

Stem Construction

Design Height Above Ftg

ft = 0.00
 Wall Material Above "Ht" = Concrete
 Design Method = SD SD SD SD SD
 Thickness = 10.00
 Rebar Size = # 7
 Rebar Spacing = 8.00
 Rebar Placed at = Edge

Design Data

fb/FB + fa/Fa = 1.140

Total Force @ Section

Service Level lbs =
 Strength Level lbs = 6,348.0

Moment....Actual

Service Level ft-# =
 Strength Level ft-# = 30,024.0

Moment.....Allowable

= 26,327.0

Shear.....Actual

Service Level psi =
 Strength Level psi = 70.0

Shear.....Allowable

psi = 75.0

Anet (Masonry)

in2 =
 Wall Weight psf = 125.0
 Rebar Depth 'd' in = 7.56

Masonry Data

f'm psi =
 Fs psi =
 Solid Grouting =
 Modular Ratio 'n' =
 Equiv. Solid Thick. =
 Masonry Block Type =
 Masonry Design Method = ASD

Concrete Data

f'c psi = 2,500.0
 Fy psi = 60,000.0

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic

Concrete Stem Rebar Area Details

Bottom Stem	Vertical Reinforcing	Horizontal Reinforcing	
As (based on applied moment) :	0.9211 in2/ft		
(4/3) * As :	1.2281 in2/ft	Min Stem T&S Reinf Area 2.880 in2	
200bd/fy : 200(12)(7.5625)/60000 :	0.3025 in2/ft	Min Stem T&S Reinf Area per ft of stem Height : 0.240 in2/ft	
0.0018bh : 0.0018(12)(10) :	0.216 in2/ft	Horizontal Reinforcing Options :	
	=====	One layer of :	Two layers of :
Required Area :	0.9211 in2/ft	#4@ 10.00 in	#4@ 20.00 in
Provided Area :	0.9 in2/ft	#5@ 15.50 in	#5@ 31.00 in
Maximum Area :	1.0245 in2/ft	#6@ 22.00 in	#6@ 44.00 in

Footing Data

Toe Width	=	4.75 ft
Heel Width	=	2.50
Total Footing Width	=	7.25
Footing Thickness	=	18.00 in
Key Width	=	0.00 in
Key Depth	=	0.00 in
Key Distance from Toe	=	0.00 ft
f'c =	2,500 psi	Fy = 60,000 psi
Footing Concrete Density	=	150.00 pcf
Min. As %	=	0.0018
Cover @ Top	2.00	@ Btm.= 3.00 in

Footing Design Results

	Toe	Heel
Factored Pressure	= 5,268	0 psf
Mu' : Upward	= 31,145	0 ft-#
Mu' : Downward	= 3,892	3,431 ft-#
Mu: Design	= 27,253	3,431 ft-#
phiMn	= 54,690	29,913 ft-#
Actual 1-Way Shear	= 41.13	22.13 psi
Allow 1-Way Shear	= 75.00	75.00 psi
Toe Reinforcing	= # 7 @ 8.00 in	
Heel Reinforcing	= # 6 @ 12.00 in	
Key Reinforcing	= None Spec'd	
Footing Torsion, Tu	=	0.00 ft-lbs
Footing Allow. Torsion, phi Tu	=	0.00 ft-lbs

If torsion exceeds allowable, provide supplemental design for footing torsion.

Other Acceptable Sizes & Spacings

Toe:
 Heel:
 Key:

Min footing T&S reinf Area	2.82	in2
Min footing T&S reinf Area per foot	0.39	in2 /ft
If one layer of horizontal bars:		If two layers of horizontal bars:
#4@ 6.17 in		#4@ 12.35 in
#5@ 9.57 in		#5@ 19.14 in
#6@ 13.58 in		#6@ 27.16 in

Project Title:
 Engineer:
 Project ID:
 Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic

Summary of Overturning & Resisting Forces & Moments

ItemOVERTURNING.....			RESISTING.....		
	Force lbs	Distance ft	Moment ft-#		Force lbs	Distance ft	Moment ft-#
HL Act Pres (ab water tbl)	3,189.4	4.50	14,352.2	Soil Over HL (ab. water tbl)	2,500.0	6.42	16,041.7
HL Act Pres (be water tbl)				Soil Over HL (bel. water tbl)		6.42	16,041.7
Hydrostatic Force				Watre Table			
Buoyant Force	=			Sloped Soil Over Heel	=		
Surcharge over Heel	=	945.0	6,378.8	Surcharge Over Heel	=	416.7	2,673.6
Surcharge Over Toe	=			Adjacent Footing Load	=		
Adjacent Footing Load	=			Axial Dead Load on Stem	=		
Added Lateral Load	=			* Axial Live Load on Stem	=		
Load @ Stem Above Soil	=			Soil Over Toe	=	2.38	
Seismic Earth Load	=	765.5	5,166.8	Surcharge Over Toe	=		
	=			Stem Weight(s)	=	1,500.0	7,750.0
Total	=	4,899.8	O.T.M. = 25,897.7	Earth @ Stem Transitions	=		
				Footing Weight	=	1,631.3	5,913.3
				Key Weight	=		
				Vert. Component	=		
Resisting/Overturning Ratio		=	1.25	Total =	6,047.9 lbs	R.M.=	32,378.6
Vertical Loads used for Soil Pressure	=	6,047.9	lbs				

* Axial live load NOT included in total displayed, or used for overturning resistance, but is included for soil pressure calculation.

If seismic is included, the OTM and sliding ratios may be 1.1 per section 1807.2.3 of IBC.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Sliding Resistance.

Vertical component of active lateral soil pressure IS NOT considered in the calculation of Overturning Resistance.

Tilt

Horizontal Deflection at Top of Wall due to settlement of soil

(Deflection due to wall bending not considered)

Soil Spring Reaction Modulus 250.0 pci

Horizontal Defl @ Top of Wall (approximate only) 0.173 in

The above calculation is not valid if the heel soil bearing pressure exceeds that of the toe, because the wall would then tend to rotate into the retained soil.

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

Project File: Typical Detail Co-04-07.EC6

LIC# : KW-06014947, Build:20.22.3.31

SWENSON SAY FAGET

(c) ENERCALC INC 1983-2022

DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic

Rebar Lap & Embedment Lengths Information

Stem Design Segment: Bottom

Stem Design Height: 0.00 ft above top of footing

Lap Splice length for #7 bar specified in this stem design segment = 40.95 in

Development length for #7 bar specified in this stem design segment = 31.50 in

Hooked embedment length into footing for #7 bar specified in this stem design segment = 14.70 in

As Provided = 0.9000 in/ft

As Required = 0.9211 in/ft

Project Title:
Engineer:
Project ID:
Project Descr:

Cantilevered Retaining Wall

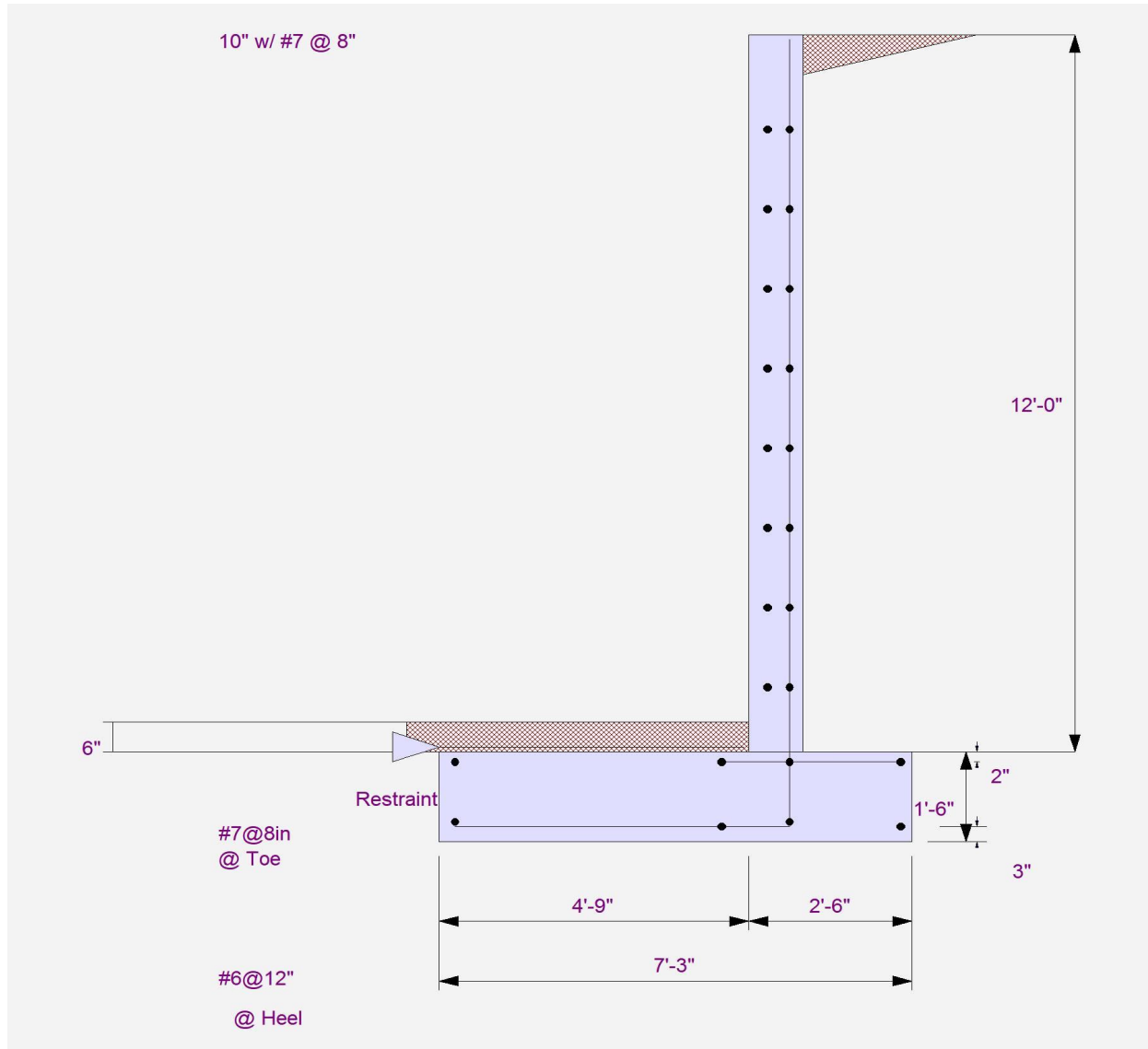
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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic



Project Title:
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DESCRIPTION: Retaining Wall Schedule 12'-0" Retaining Wall w/ Slab and Traffic and Seismic

